SELECTIONS FROM THE RECORDS OF THE BOMBAY GOVERNMENT.

No. XLVII.-New Series.

REPORT ON A PROJECT

FOR

THE SUPPLY OF WATER

TO THE

POONA CANTONMENT.

ВY

CAPTAIN PHILIP LEWIS HART,
BOMBAY ENGINEERS.

Waith Plans and Sections In a Separate Case).

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REPORT

ON THE

SUPPLY OF WATER TO THE POONA CANTONMENT,

BY

CAPTAIN PHILIP LEWIS HART,

BOMBAY ENGINEERS.

"To select and mature the most promising of the various schemes either already submitted, or which he (the Officer deputed on the duty) might himself find it expedient to suggest."—Extract from Chief Engineer of Public Works' Letter to Government, No. 10211 of 1856, dated 20th December, paragraph 16.

- 1. The following are the various schemes which have been already submitted for supplying the Poona Cantonment with water; but they are all, more or less, so mixed up with the supply to the City of Poona, that I have been unable to separate them, although the former is the subject with which I have to deal in this Report.
- 2. Captain Jacob's proposal (dated 4th December, 1851) was for increasing the supply of water to the City of Poona by removing the supposed deposit from the Lower Kartriz Tank.
- 3. Captain Graham's projects (dated 31st October, 1851) for supplying the City of Poona and the Cantonment are as follows:—For the City of Poona. 1st. By increasing the supply of water at the Lower Kartriz Tank in different ways. By raising the wall of the Kartriz Tank, at an estimated cost of Rs. 18,402. For increasing the supply in the Kartriz Tank, by conveying the water from the upper to the lower tank; and by the construction of various masonry aqueducts to catch the water from a certain Nullah and supposed spring, at an estimated cost of Rs. 66,704. 2nd. By constructing

sluices at the Sangroon, Kanapoor, and Gorleh Dohoos, or natural lakes, in the Mootta River twelve miles west of Poona, and allowing the water therefrom to flow down the bed of the river to Poona: this project does not appear to have been estimated. 3rd. By increasing the supply of the Nana aqueduct: this also was not estimated .- For the Cantonment of Poona. 4th. By increasing. the supply of Rastiah's aqueduct. in different ways. By extending the same at an estimated cost of Rs. 2,725. By doming over two wells and conveying their water to the aqueduct, at an estimated cost of Rs. 30,000. 5th. By obtaining possession of the Chowdrey's aqueduct, and throwing the supply therefrom into Rastiah's aqueduct, and, in return, supplying that portion of the City of Poona called Vetal Peth from the Kartriz aqueduct. To collect the overflow of Rastiah's aqueduct during the monsoon into two reservoirs, one in the Civil Lines and the other near the Collector's Office, at an estimated cost of Rs. 3,755 and Rs. 4,000. 6th. If the Chowdrey will not give up his aqueduct, to complete the supply to the Cantonment by a new Bund above the village of Great Kondwah, and building a new aqueduct from the Kondwah Nullah, parallel to the Chowdrey's aqueduct; and by sinking shafts in different directions in the hot weather, with the view of finding out springs, and conducting them to the main aqueduct, at an estimated cost of Rs. 1,26,886.

- 4. Captain Kilner's project (dated 15th January, 1852) for supplying the Cantonment and a portion of the City of Poona, consisted of bringing the water from the Upper Kartriz Tank, at an estimated cost of Rs. 1,11,887, by iron pipes into the cistern near St. Mary's Church; from this cistern to one in the Bowanee Peth, and a branch to the Malcolm Tank in the Bazar.
- 5. Vickajee Meerjee's project (dated 9th October, 1851) was for bringing water from a spring at Duncowree to the Bowanee Peth in the City of Poona, estimated by him at Rs. 50,000, and by Major Kilner at from Rs. 60,000 to Rs. 70,000.
- 6. To complete the supply from the Jamsetjee Bund Water-works, at about two lacs and forty-one thousand gallons daily for the Cantonment, Bazar, and suburbs, by steam power, at an outlay of Rs. 1,60,122.
 - 7. Captain Berthon's scheme (dated 10th March, 1853) was to

construct an under-ground Bund of masonry across the Kondwah Nullah, for the supply of the Cantonment, with an arched gallery, to be incorporated with the masonry of the Bund, to act as filterer and receiver. From the receiver, the water is to be carried in a 15-inch main to a central cistern, or to some convenient position in or near the Horse Artillery Lines, whence it is to be led by branch pipes to the Barracks, and other public places of resort requiring a supply, at an estimated cost of Rs. 1,23,910.

8. Mr. Reeves, the Revenue Commissioner, recommends the con-

Mr. Reeves' letter, No. 1875 of 1855, dated 3rd July; also paragraph 15 of No. 3220 of 1854. Military Board's letter, No. 1023 of 1855, dated 5th February. Chief Engineer of Public Works' report, No. 10211, dated 20th December, 1856.

struction of a dam across the Mootta River between Sangroon and Gorleh, where the water of the river is retained in extensive natural reaches, called Dohoos, with iron pipes from them into the Cantonment and City; and he suggests that some of the water could be sold for irrigation.

- 9. Mr. Gerrard, Civil Engineer, proposes a scheme (dated 18th October, 1856) for the supply of the Poona Cantonment only, by damming up the Ambeygaum Valley, and leading the water in a conduit of earthenware pipes to a distributing reservoir at the back of the Hospital to the Wanowree Barracks, and thence by iron pipes to the various localities in the Camp and Cantonment Bazar, at an estimated cost of Rs. 2,48,917.
- 10. Before offering any remarks on the various projects above mentioned, I will premise, that a population of forty thousand (in round numbers; see Appendix, Letter A) have to be supplied daily throughout the year with twenty gallons of water per head, which is the minimum allowed for in England; and that the question of a supply of water to the Poona Cantonment involves the equally important one of a thorough drainage and sewerage of the whole site; a point, however, upon which I have not been called on to report, but which must be borne in mind in connection with any efficient project of water supply. On this latter consideration alone I should have much preferred a calculation of thirty gallons per head; but, as it might perhaps be considered that I had exaggerated the wants of the Camp, in assuming the requirements at that rate, I will retain the former quantity of twenty gallons, being, I believe, as before observed, the minimum supply usually allowed for in England.

- 11. The quantity of water then required for forty thousand people, at twenty gallons per head, amounts to a supply of eight hundred thousand gallons daily, or two hundred and ninety-two million gallons yearly,—taken, in round numbers, at three hundred million gallons; the water being delivered into a distributing reservoir of such an elevation as to be able, when required, to flush every drain and sewer in camp which may hereafter be constructed.
- 12. The above are the main standards by which I consider all the foregoing projects must be compared, although it must be borne in mind that, at the time they were framed, the population was nothing like the present number of forty thousand; that the quantity, even of twenty gallons per head daily, would have been considered much in excess of the actual wants, and that the subject of the sewerage of the Camp had not become so important a matter as at present, when the population has so much increased. This is clear, as the drainage of the Camp in connection with the water supply is not noticed, that I can discover, in any of the Reports hitherto submitted.
- 13. This does not concern the supply to the Cantonment in the least, as it refers to the Lower Kartriz Tänk, a work specially belonging to the Municipality, for increasing the supply to the City of Poona; but I allude to it for two reasons, which I cannot pass over without remark. The first, that Major Kilner considered the filling up by deposit of the Lower Kartriz Tank an illusion; and the second, because it was considered, both by the same Officer and Captain Graham, that raising the Bund wall of the tank was a cheaper method of obtaining an extra supply of water than by excavating the débris out of it.
- 14. I can observe that, during the last seventeen years, the deposit in the lower tank has increased considerably, and is easily accounted. for.* Portions of the retaining walls to the artificial supply channel

^{*} When I was in Poona in 1839, as Assistant Superintendent of Roads and Tanks, one of these walls had been rebuilt by Lieutenant Wood, and had been carried away during the first floods which came down the supply channel. It could not be rebuilt until the monsoon moderated, and doubtless, during the period the wall remained unbuilt, large quantities of deposit found their way into the bed of the tank. Whether this has happened since I cannot say, but the appearance of different portions of the wall favours the idea that they must have been rebuilt at some time or other.

have at different periods been washed down, generally of course at the first heavy floods of the monsoon; and through the gap so created, and which could not be rebuilt during the rains, large quantities of mud have been brought down from the hills, and deposited in the bed of the tank. With respect to raising the Bund of the tank being cheaper than excavating the mud out of it, it certainly may be, if the mere excavation of the mud by manual labour from the bed of the tank, and depositing it on the banks, be intended; but, if properly managed and cared for, immense quantities might be yearly got rid of by washing it out in a liquid state through the sluices at a small cost. The evil has probably now gone too far for any arrangement of this description being of much use.

- 15. In the year 1841, from the 3rd of May to the 11th of June, 171 prisoners from the Poona Jail were encamped on this work, and, with the aid of some free labourers, a large quantity of the deposit then formed was washed out through the sluices in a liquid state. this period, as far as I have been able to ascertain, no attempt whatever has been made to get rid of any of the deposit, beyond yearly opening the sluices to allow of the first floods of the rainy season to pass off, and to close them when the feeding channel runs clear. I rather doubt whether even this has been regularly attended to. ing been some years ago an assistant to the Superintendent of Roads and Tanks in this Collectorate, and had something to do with this work, I am enabled to offer an opinion regarding the deposit formed in it, and which is still in progress in my opinion; and I have no hesitation in saying that it is rapidly filling up, like the one above it, and that it is, from long and continued neglect, becoming inefficient as a storage reservoir for water.
- 16. Of the six projects submitted by Captain Graham, three refer to the supply of water to the City of Poona, and three to the Camp. The one relating to the bringing water from the Sangroon, Kanapoor, and Goreh Dohoos I shall hereafter offer a few remarks on; at present I will confine myself to the notice of those referring to the Cantonment supply. From a careful inspection of the works alluded to by Captain Graham, it appears to me that not one, or all the projects taken together, as regards the Chowdrey's and Rastiah's aqueducts, collecting their overflow in the monsoon, extending or uniting them, or by

doming over the wells mentioned, and conveying their water to these works, would give the supply now required, viz. eight hundred thousand gallons daily throughout the year; and supposing the supply to be sufficient, I much doubt if the water could be delivered by gravitation at such an elevation as to supply every public building in the Camp, or be made available for flushing the sewers and drains to be hereafter laid out.

17. Captain Graham's proposal regarding these two works, Rastiah's and the Chowdrey's aqueducts, was as follows (see his letter No. 2300 of 1853, dated 8th August, paragraph 2):—

"To make the matter at issue as clear and concise as possible, I must remind the Board of the nature of my proposal, which is to supply the whole of the City of Poona from the Kartriz aqueduct, and the Cantonment of Poona from the joint supply obtainable from Rastiah's and the Chowdrey's aqueducts."

Captain Graham then goes on to observe, whether the supply obtainable from these two aqueducts, added to the probable increase from new sources, will suffice; and compares his scheme with those of Major Kilner and Captain Berthon. I have taken below the most favourable results anticipated by Captain Graham from the proposed improvements to these aqueducts, which are as follows:—

19th	March	1	3,38,798	gallons.
31st	do.		2,53,496	,,
	April		2,11,702	,,
	May		1,32,345	,,

After a long and somewhat tedious correspondence on the subject of these two aqueducts, the owners, Rastiah and the Chowdrey, refused to give them up to Government. (Mr. Reeves' letter, dated 15th February, No. 510 of 1856.)

18. The only other project remaining to be noticed of Captain Graham's, is that for constructing a dam above the village of Great Kondwah, and building a new aqueduct parallel to the Chowdrey's. As regards the locality for supplying the Camp, nothing could be better; its situation and distance are admirable. The levels also suit well, as the proposed site for a dam is 135.75 feet above the old Hospital Compound in front of the solitary cells, one of the

highest points in the Camp. But, if the general configuration of a valley for impounding water ought to be deep, with steep sides and surrounded by lofty hills, then the Kondwah valley does not, certainly in my opinion, answer this description. It is flat and shallow, and only bounded by lofty hills at its head, without any spurs of the slightest consideration from the main range to form its sides. I think therefore that this valley, from the above cause, is extremely ill adapted for impounding water, although in it there is a large quantity of underground drainage, fifteen or twenty wells being worked, during the hot weather, on its banks for the Pawn gardens.

19. To show how extremely shallow a valley it is generally, I would remark that the dam proposed by Captain Graham for a reservoir above the village of Great Kondwah, when raised to its greatest possible height, is only 32 feet, with a length of 1,400 feet. The estimated supply daily for the hot weather comes to seven hundred and ninety-five thousand gallons for three months, one-third of the actual contents having been taken for evaporation; but, in a flat shallow work of this description, the surface evaporation would in all probability be much greater.

20. I will now proceed to notice Major Kilner's project for supplying the Camp and a part of the City Major Kilner's project. of Poona with water from the Upper Kartriz Tank. Major Kilner reckons the Camp population at twentyfour thousand, and the quantity to be allowed for per head at eight gallons. The quantity required, therefore, is one hundred and ninetytwo thousand gallons daily, to which is to be added, for the Horse Artillery horses about four thousand gallons, and for Sir Jamsetjee Jejeebhoy's premises five thousand gallons, making altogether two hundred and one thousand gallons. The Upper Kartriz Tank he calculates to contain forty-seven million nine hundred and fifteen thousand gallons, exclusive of all water in the deposit, with which it is about two-thirds filled up, which for 182 days, or half the year, affords two hundred and forty-one thousand two hundred and ninetyone gallons daily. In this no account is taken of the water in the deposit, or "black clay," supposed to consist of 20,430,956 cubic feet, of which probably, Major Kilner states, one-sixth is water, giving twenty-one millions, two hundred and eighty-two thousand, two

hundred and forty-three gallons in this medium. This is in addition to the daily supply above.

- 21. The total supply therefore, including that in the "black clay" (which has to be got at), amounts to sixty-nine millions, one hundred and ninety-seven thousand, two hundred and forty-three gallons, which would give daily throughout the year a supply of one hundred and eighty-nine thousand five hundred and eighty-one gallons, or daily for half the year three hundred and seventy-nine thousand one hundred and sixty-two gallons. Since Major Kilner's report was written, the Camp population has increased from twenty-four thousand to forty thousand, and the rate per head daily, which he calculates at eight gallons, is, without doubt, much under the mark. It is therefore very apparent that his project would not supply the present demand. I consider that Major Kilner has much underrated the evaporation. Over an area of 1,916,600 square feet, of an average depth of six feet, an extensive shallow expanse, the evaporation is only taken at one-third, or two feet of depth over the whole surface; whereas, with very little of the supply drawn off, the evaporation alone would go considerably towards emptying such a reservoir.
- 22. I feel satisfied, from having carefully watched this tank during the last hot season, that it cannot be depended on, in its present state, for a full and constant supply of water throughout the year. It is also to be observed that, whatever water is taken from the Upper Kartriz Tank interferes with the supply to the lower tank. This work, in fact, as much belongs to the municipality of the City of Poona as the lower tank does; and as Government are precluded (Government Resolution dated 19th September, 1855), by the express orders of the Honorable Court, from expending public money in order to provide an enlarged supply of water to the City of Poona, even supposing that the upper tank contained sufficient storage room for the supply of the Cantonment, it does not appear to me, under existing circumstances, how it could possibly be appropriated to such a purpose.
- 23. With reference to this Lower Kartriz Tank, which is fed to a great extent by the upper one, I would observe that the following Government buildings in the City of Poona are supplied with water from it:—

Name of Building.		Probable No. of Governme servants using the water			iter.										
1.	Shunwar I	Palace	٠.											173	3
2.	Boodhwar	do.										٠.		574	Į.
	Nana's														
4.	Jail										٠.			413	3
								T	'ot	al			:	1,239	

- 24. As regards the water held in suspension by the deposit in the Upper Kartriz Tank, the quantity of it is of course a surmise; and as the difficulty of getting at it by simple means Major Kilner pronounced to be almost insurmountable, in so far as it affects that Officer's calculations, it was very properly omitted from his estimate of supply. I consider that Captain Graham has suggested the only way of getting at it, viz., by sinking deep shafts and connecting them by galleries or cuts covered over, extending over the whole bed of the deposit, and leading these channels into one or more main channels, as might be found convenient, through the Bund. What the actual supply would be by tapping this extensive water-bearing débris, it is impossible to say. If a very frequent inspection of this work, and a mere practical opinion be worth anything, without figures, I should say its capabilities of supply were very considerable, but that no satisfactory information could be obtained on this point, without going to some actual experiment, which, in all probability, would be attended with considerable expense.
- 25. I have very frequently inspected the little valley of Dun-Vickajee Meerjee's Report. cowree, from which Vickajee Meerjee proposed obtaining a supply of water for a portion of the City of Poona; and, for its extent, I am of opinion that its capabilities are very considerable, but of too limited a character as an independent supply, although in any scheme which would admit of employing it as a subsidiary, it would, I am sure, be likely to prove a very valuable one.
- 26. There is nothing to observe regarding this work, except that it has, I believe, been abandoned as a principal source of supply, owing to the polluted nature of the water, receiving as it does, at no great distance off, the whole sewerage from the City of Poona.

27. This project is for what Captain Berthon terms "an underCaptain Berthon's scheme. ground Bund across the Kondwah valley stream." There is no doubt that it would add most materially to the supply of the present Chowdrey's aqueduct. I am of opinion that it is the only work of the sort adapted to the nature of this valley. As regards the supply of water to be derived from it, it is impossible to conjecture even, but I should be inclined to doubt much if 800,000 gallons daily could be furnished from such a contrivance, in the locality indicated.

28. These projects are the same, for bringing the water confined in

Captain Graham's and Mr. Reeves' project for bringing water from the Sangroon Dohoo. the Sangroon, Kanapoor, and Goreh Dohoos, or pools in the bed of the Mootta River, for the supply of the City and Camp of Poona. I carefully inspected these pools, which are

fifteen or sixteen miles south-west from Poona; their extent is as follows:—

Names of Dehoos.		by Major Kilne nd November,		Measured by Captain Hart in February, 1857.			
Traines of Denous.	Length.	Width.	Greatest Depth.	Length.	Width.	Greatest Depth.	
	M. F. Yds.	Feet.	Feet.	M. F. Yds	Feet.	Feet.	
Goreh Dohoo	07 38	222 to 425	97	0 6 180	250	5	
Kanapoor Dohoo	0 7 93	256 to 269	113	1 0 100	250	6	
Sangroon Dohoo	1 1 186	180 to 310	174	1 2 30	230	104	

29. One set of levels was taken by Mr. Gerrard, C.E., in November, 1855, and the difference of level, between the top water of the Sangroon Dohoo (the uppermost) and the top of the centre of the Jamsetjee Bund, was found to be 94.59 feet. The same levels were taken very carefully by Surveyor and Builder Venaik Bhickajee* in February, 1857, and was found by him to be 81.74 feet, the difference being caused, in all probability, by the fall of the water between November and February, and, deducting the depths at about these

Poona. The difference of level between the top waters of these two pools was at the same time found to be 26.13 feet,—a considerable fall in so short a distance.

94·59 17·75	Ir. Gerrard's. Depth measured by Major Kilner.
76.84	
81·74 10·50 71·24	Venaik's. Depth measured by Venaik.

periods, would make the bottom of the Sangroon Dohoo (the uppermost) 76.84 feet higher than the top of the Jamsetjee Bund as levelled by Mr. Gerrard, C.E., and 71.24 feet as levelled by Surveyor and Builder Venaik Bhickajee. Mr. Gerrard, C.E., made no measurement of the depth when he took the levels in November 1855. I have therefore assumed the depth to be that taken by Major Kilner in October and November,

1851, which will account for the difference in the two sets of levels, both of which were taken from the top waters, and which, of course, are constantly fluctuating. Again, in measuring the greatest depth of such an extensive sheet of water as the Sangroon Dohoo, differences must unavoidably occur; however, I think that these levels, taken by different individuals at different times, are sufficiently near for all the practical purposes of determining if water can or cannot be delivered in the Camp of Poona by gravitation from these natural pools in the bed of the Mootta River. I will, therefore, take the mean of the two sets of levels $\frac{76\cdot84}{2} + 71\cdot24$, and consider the bottom of the Sangroon Dohoo to be 74·04 feet above the top of the Jamsetjee Bund, about the centre of it.

30. From careful levels* taken between fixed points in the Camp of Poona, and the top of the Jamsetjee Bund, the bottom of the Sangroon Dohoo above the same point, viz. 74 feet, is found to correspond with points in the Camp of Poona, as follows:—

Corner of His Excellency the Commander in Chief's compound; exactly.

The Sapper and Miner Lines; the point is two feet lower.

Front of the Ghorepoorie Barracks; all of which are situated in the lower parts of the Cantonments. Points in the new Wanowree Barracks are 164.9 feet and 176 feet above the top of the Jamsetjee Bund, and in the Horse Artillery Lines 169 and 152 feet above the same point. I trust that these levels sufficiently prove that water cannot be delivered from these Dohoos to all parts of the Poona Camp

^{*} I am indebted to the kindness of Mr. Coke (in charge of the Engineer School) and his pupils, for this assistance.

by gravitation. There is no necessity to remark on the immense distance they are off, some sixteen miles, when other sources are comparatively close at hand.

31. This valley is situated about five miles south-west of the City

Mr. Gerrard's (C.E.) scheme for damming up the Ambeygaum Valley.

of Poona, across the mouth of which Mr. Gerrard proposes to throw an earthen embankment, leading the water from the reservoir so formed, by a deep cutting with a consider-

able fall, across the small plateau between the Kartriz and Ambeygaum valleys, and continuing the conduit, following the sinuosities of the hills to a distributing reservoir in rear of the Hospital, old Wanowree Barracks, from which reservoir a system of earthenware piping, set in grout, is to convey the water to a number of open tanks situated in different parts of the Camp. This latter was, I believe, altered by Colonel Scott, the Superintending Engineer Central Province, to iron piping.

- 32. The quantity of water thus impounded, Mr. Gerrard calculates, will, making ample allowance for evaporation, amount to 57,192,400 cubic feet, or 357,452,000 gallons, which would give daily throughout the year at the rate of 979,323 gallons; more, by 179,323 gallons, than I have assumed the requirements to be.
- 33. I have now, I believe, noticed all the various schemes for one of the works, and examined them many times; I have also examined carefully the neighbourhood of Poona for other sites. I have nothing new to suggest, as every available situation is more or less occupied; and I have no hesitation in pronouncing that I consider Mr. Gerrard's scheme best adapted to present requirements. As far as I can see, it does not interfere with any real or supposed rights on the part of the municipality of Poona, or of individuals. The gathering ground is good and ample; there are no engineering difficulties whatever in the works; and the valley possesses all the requisites for an impounding reservoir, in a most favourable degree; and as Mr. Gerrard's designs for the details of this project have been noticed as objectionable by the Chief Engineer of Public Works (Chief Engineer of Public Works' letter to Government, No. 10211 of 1856, dated 20th December) in several particulars, having selected it as the most promising of the various schemes submitted, I now proceed to mature

it, keeping in view the Chief Engineer's remarks above alluded to. At the same time, I will not mislead Government by allowing them to suppose that it can be carried out for a small outlay. Supplying forty thousand people with pure drinking water from a distance of six or seven miles, and delivering it at convenient situations over a large and extensive Camp, besides providing storage room for a head of water to flush sewers and drains, in addition to the main reservoir, cannot be carried out for a small amount.

34. The general explanation of the scheme now submitted is as fol-

A general explanation of the scheme submitted by Captain Hart. lows:—The construction of an earthen dam across the gorge of the valley near the village of Upper Ambeygaum, 1,270¹/₄ feet in length, its greatest height above the bed of the

From the reservoir so formed, the water stream being 59.86 feet. will be led by either an iron conduit pipe or masonry aqueduct, whichever may be the most economical, to the most favourable point for crossing the small plateau of the spur dividing the Kartriz and Ambeygaum valleys. Through this portion a tunnel will be driven 2,781 feet long, opening at the head of the Duncowree Valley. course then for either pipe or aqueduct will be down this little valley for some short distance. If by pipe, it will run almost straight to a distributing reservoir to hold one or two days' supply, near the solitary cells in the Camp of Poona; and if by an aqueduct, before reaching the mouth of Duncowree Valley, it will branch off, running slightly up the Kartriz Valley, crossing the Kartriz aqueduct, round under the village of Beebee Warree, and thence following the sinuosities of the hills to the distributing reservoir above mentioned. If by a masonry aqueduct, the length will be 7m. 6 fur. 65½ yds.; or, by an iron pipe, it will be 5m. 1 fur. 943 yds., the total fall from the bottom of the reservoir being 80.25 feet. The distributing reservoir, as above observed, will either contain one or two days' supply, and from it the water will be conveyed to every part of the Camp by iron pipes, with a stand-pipe at each cistern. The situation of the cisterns, their number and capacity, will correspond with the wants of the neighbourhood in which they are located.

35. The following Plans accompany this Report. All have been plans accompanying the surveyed and prepared affesh, without the slightest reference to those submitted by Mr.

Gerrard, C.E., which, from their description, I imagine were intended to be of an entirely preliminary nature.

- No. 1.—Trigonometrical survey of the Ambeygaum valley, and country between it and Poona.
- No. 2.—Plan of the embankment, with longitudinal and transverse sections, &c. &c.
- No. 3.—Contoured Plan of the reservoir, with site of the waste weir and cut, &c. The contours being taken at every four feet of vertical distance.
- No. 4.—Plan, elevation, and section of the gangway, with details, &c.
- No. 5.—Plan, elevation, and section of the inlet tower.
- No. 6.—Various details, with a plan and section of a two days' distributing reservoir.
- No. 7.—Various details, with a plan and section of a one day's distributing reservoir.
- No. 8.—Plan and longitudinal sections of the whole course of an iron pipe and masonry aqueduct, with details, &c.
- No. 9.—Plan of the Cantonment and Lines at Poona, showing the proposed distribution throughout the Camp.
- 36. The valley of Ambeygaum is situated about five miles southwest of Poona, being one of those formed by the spurs which run out from the Singhur range of hills. Its southern limit is very

lofty, being the main range itself, while on the east and west sides well-defined and lofty spurs run out. The general configuration of the valley possesses, I am of opinion, every requisite as a drainage area for collecting water. It is particularly deep and precipitous at its upper end, and flanked by lofty hills. At its lower end, near the village of Upper Ambeygaum, the mouth contracts, and it is at this point I propose constructing the earthen dam. I believe it to be the very identical spot, or very nearly so, selected by Mr. Gerrard. Excellent material for its construction abounds on the spot.

37. In paragraphs 10 and 11, and from the letter in the Appendix marked A, which was framed with much care, it is stated that the population of the Camp and Bazar, including Horse Artillery, horses, &c., amounts to 37,682, which, in

round members, may be assumed at 40,000, and with the allowance of twenty gallons per head daily, the minimum allowed for in England, 292,000,000 gallons will be the annual requirement; but, to allow a safe margin, for the reasons stated in the concluding part of this paragraph, 300,000,000 gallons per annum may be assumed as the quantity required.

	•		Gallons.	Cubic Feet.
Per	annum,	required	 300,000,000	48,154,093
11	day,	,,	 821,917	131,929
11	hour,	57	 34,246	5,497
,,	minute,	,,	 570.77	91.61
"	second,	"	 9.51	1.52

Ample allowance having, I think, been made in the calculation, for the remaining purposes of waste, leakage, flushing sewers, watering camp roads or public trees, and especially for future requirements to the Railway terminus, and extension to the Civil Lines, the surplus for which amounts to 24,921,400 gallons yearly, or daily to 68,277 gallons, equivalent to supplying 3,414 individuals with 20 gallons per head daily, or about nine per cent. above actual present requirements.*

- 38. From a most careful survey of this valley, the catchment basin, or gathering ground for the reservoir, contains an area of 4 sq. miles, 232 sq. acres, 22,680 sq. feet; or, 2,792 acres, 22,680 sq. feet.
- 39. From the Appendix marked B, containing observations of the Calculations of the rain-fall at Poona for the last twelve years, made at the Staff and Civil Hospitals, the average over that period is 25 inches 89 cents. The fall of rain, however, amongst the hills of the Ambeygaum Valley is, in all probability, much greater. The minimum fall extending over the same period is 14 inches 78 cents. As 640 acres equal a square mile, and one acre is equal to 43,560 sq. feet, a fall of one inch of rain is equal to 3,630 cubic feet per acre, and $3,630 \times 640 = 2,323,200$ cubic feet

^{*} It was partly with a view of providing at any time an increased supply that the pipe and aqueduct were led down the Duncowree Valley. Cuts in this valley, connected with the main conduit, would, I am of opinion, furnish a large additional supply at any time.

per square mile. Therefore a fall of twenty-five inches will be 58,080,000 cubic feet per square mile; and for four square miles 232,320,000 cubic feet, or 1,447,353,600 gallons. With the minimum fall of fourteen inches for four square miles, the amount will be 130,099,200 cubic feet, or 810,518,016 gallons, of which only 6-10ths may be considered available for the supply of the reservoir; making,. with the average fall of twenty-five inches, 868,412,160 gallons, and with the minimum fall of fourteen inches, 486,310,809 gallons;—in the first case, the supply being nearly three times as much as is actually required; and with the minimum fall, one and a half times more. I have assumed 6-10ths of the rain-fall as the available supply for the reservoir, as being the quantity usually allowed for, I believe, in England.

40. Attached to this Report, in the Appendix lettered C, is a Result of gauging, dur-

ing the last hot season, the Great Kondwah Nullah, the back of the Upper Kartriz Tank, and the Ambeygaum Nullah.

return of the water gauged at the Great Kondwah Nullah, the back of the Upper Kartriz Tank, and the Ambeygaum Nullah, from the 24th February to the 8th June. 1857; the general result of which is as follows:---

Gallons.

The Great Kondwah Nullah supply was . . 1481 per hour. 300

The Upper Kartriz Tank do. ,, The Ambeygaum Nullah do. .. 1179

A sufficient approximation, I think, that the first and last, affording such a supply during the hot season, are amply sufficient for supplying the Camp, while several heavy floods passed down these streams and were lost; and as regards the Upper Kartriz Tank, sufficient dependence is not to be placed on it, in its present condition, as a means of supply.*

41. Having shown that the gathering ground is more than ample for the supply of the estimated requirements. Storage capacity of the I proceed to notice the storage capacity of proposed reservoir. the reservoir. The following Table shows the contents of different heights, the contours having been taken at 4 feet vertical distance apart:—

^{*} At this time, 3rd August, 1857, the tank is nearly dry.

Depth of Reservoir.	Area in Acres and Square Yards.	Cabic Feet.	Gallons.
Feet.	Acres. Sq. Yds.		
-1	4 2593	790,316	4,923,668
8	8 1041	2,221,714	13,841,278
12	13 2436	4,574,554	28,499,471
16	17 4670	7,704,774	48,000,742
20	23 4246	11,865,170	73,920,009
24	28 2732	16,842,272	104,927,354
28	34 952	22,800,728	142,048,535
32	40 1370	29,819,666	185,776,519
36	45 2798	37,761,220	235,252,400
40	51 861	46,678,598	290,807,665
4.1	63 3028	57,764,750	359,874,392
48	77 2979	71,288,504	444,127,387

So that, with the dam at the 40-feet high contour, the reservoir will contain 290,807,665 gallons, sufficiently near the requirements assumed, of 300,000,000 gallons, for all practical purposes.

42: Colonel Scott, the Superintending Engineer of the Central

Evaporation. Col. Scott's letter, No. 605 of 1856, to the Assistant to the Chief Engineer of Public Works, dated 7th February. Province, from experiments made on different tanks, states that half an inch per day would be an ample allowance to make. This quantity during the eight dry months would amount to 10 feet annually. In assuming,

therefore, in the Deccan the evaporation at eight feet, I think a safe allowance has been made, as at least two feet may be deducted from Colonel Scott's experiments for casual showers at the commencement of the cold and towards the close of the hot season. The supply therefore of the reservoir, at 48 feet, is 444,127,387 gallons, which is the proposed top water; 4 feet above that again being taken for the top of the embankment; and the bed of the Nullah below the level of the first contour being 7.86 feet, will make the total height of the embankment, in its highest part, that is, from the bottom of the bed of the stream to the top of the dam, 59.86 feet.

- 43. I will now proceed to give a description of the principal works' required, in the following order: -
 - 1. Embankment.
 - 2. Waste weir.
 - 3. Cut to carry off first floods.

- 4. Inlet tower and filter.
- 5. Gangway.
- 6. Aqueduct, pipe, and tunnel.
- 7. Distribution reservoirs.
- 8. Camp distribution.

44. The following are the dimensions of some of the principal embankments for impounding reservoirs in England and America:—

Names of Works.	Height above top of Water.	Width of Embank- ment at top.	Inside Slope.	Outside Slope.	Remarks.
Longdendale Reservoir	-1	27	3 to 1	2 to 1	
Crowden do	4	15,	3 to 1	2 to 1	* With a berm 5
Albany Works	8	10	2 to 1*	2 to 1	* With a berm 5 feet wide on the in- < side slope; the re- mainder of the slope
Brooklyn do	5	20	3 to 1	2 to 1	3 to 1.

I have therefore adopted the breadth of the dam at top to be 20 feet, inside slope 3 to 1, and outside 2 to 1; with a puddle-wall of clay in the centre of the dam 8 feet wide at top, increasing in width at the rate of 4 feet for every 10 feet of depth; and the top of the dam to be 4 feet above the top water. Careful excavations were made along the line of the intended embankment, the bottom of which is shown by the dotted line coloured yellow in the longitudinal section in Plan No. 2, which line denotes the probable bottom of the puddle trench along the whole length. At the foot of the embankment on the upstream side, a small dwarf wall of rubble masonry will be built, for the pitching which protects the inner slope to abut against. The embankment will be formed in regular layers, well watered and rammed, sloping inwards from both faces. Some slight difficulty may be apprehended in draining the trench in its deepest part, to receive the puddle, but excellent material exists on the spot for the formation of both puddle and embankment. The length of the dam will be 1,270½ feet, its greatest height 59.86 feet, the reservoir having an area of 77 acres 2,979 square yards.

45. From a careful observation of most of the remains of tanks which I have seen in this country.-I, of 2. Waste Weir. course, allude to those formed by earthen mounds, including the ruins of many in the Nundoorbar Talooka of the Kandeish Collectorate,-the embankments almost invariably appear to have failed from having either no waste weir at all, or one of very limited dimensions. The reason of the failure of the embankment of the Kussoordee Tank arose, I believe, entirely from this cause. It is very clear that a sufficient outlet for the rapid escape of heavy floods in works of this nature, in a country where they are occasionally so sudden, is obviously necessary. It will scarcely be credited with what rapidity and suddenness these mountain streams fill, but by those who have actually witnessed it. In May last, the surveyor was surveying the upper portion of the Ambeygauin Valley, and had only just time to remove the chain and field compass out of the Nullah, when it came down, warning having been given in time by a cultivator at work in his fields at some distance higher up the valley. When the flood came down, the man described it like a wall of rolling water, and I can affirm the description to be perfectly true. therefore, on the above accounts, to be strongly impressed with the absolute necessity of providing an ample outlet for the surplus water, altogether independent of the artificial cut shown in Plan No. 3, which will be hereafter noticed. From an inspection of this drawing, it will be seen that the ground is favourably situated for the construction of the waste weir round the eastern end of the embank-I have, accordingly, provided for a width of 100 feet, the channel being 841 feet long. With a depth of water of one foot over this channel, the fall of water, according to a formula of De Buat, would be per hour 7,516,968 gallons.

3.—Cut to carry off first a river, or an embankment to dam up a stream, wherever, in fact, running water is opposed to any extent, that silting-up does not follow as a matter of course, and of time only as to its extent. This can be retarded in some degree, by the adoption of such a form of dam that the silting it up may be locally regulated; or by openings in the dam, through which, to a certain extent, it can be got rid of; but no practical means that I have yet seen can wholly prevent it. For instance, supposing,

instead of the carthen mound for the work, that a masonry dam was substituted, and the masonry was perforated everywhere with openings for sluices, the silting-up still takes place at the sides of the reservoir and in the spaces between the openings, for there must be some portion of dead wall in the dam. We have, in fact, practical examples before our eyes everywhere, of works of this nature filling up with deposit.

- 47. In all tanks formed by damming up running streams,—and the same cause is operating with respect to rivers, take the Khandeish river dams, for instance,—the filling up is doubtless caused by the first heavy floods of the monsoon rains, bringing down with them in suspension portions of the surfaces of the arid hills. This silting-up is in progress, more or less, in every situation under similar circumstances; yet, in moister climates, where vegetation prevails, the progress is of course not so rapid as in dry ones, where, for eight months of the year, the burning sun of a cloudless sky has full play on the parched-up soils. This being immediately succeeded by heavy tropical rains, portions of the surfaces of the hills are annually washed off, and the detrition deposited wherever the course of the running water is obstructed.
- 48. Whatever advantages may be attached to the sluices formed in a newsonry dam, by keeping them open and allowing the first floods of the monsoon to pass through, to disturb the consistency of an extensive earthen mound by any perforations of this nature, however strongly such may be protected, would, in my opinion, be extremely unadvisable.
- 49. To prevent therefore, as much as possible, the detritus from the hills being brought down during the first heavy floods of the monsoon, and deposited in the reservoir, I propose to divert the feeding channel by an artificial cut, with strong masonry walls across the main and minor streams. This cut will pass round the eastern end of the embankment, as shown in Plan No. 3. Openings will be left in these cross walls, with planks to lift up and down, through which the clear water may be permitted to enter the tank, when the first heavy, muddy floods have passed off. The Lower Kartriz tank is provided with a channel of this description, and it is in all probability owing to this that the tank has not as yet wholly filled up. In such an artificial channel as that recommended, it is particularly desirable that the masonry works across the main and minor streams should

be of strong section, and not likely to fail; or the opening caused by the failure of one wall would allow so much of the deposit to fill the tank as could in all probability never be wholly removed, even supposing sluices were provided; and where there are none, it would remain, and never be got rid of at all.

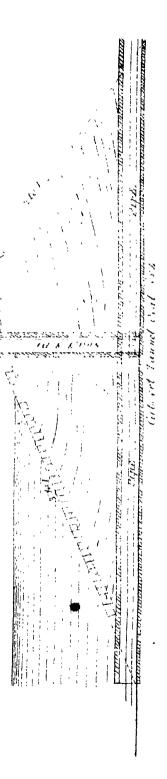
- 50. My idea is, although no provision has been made in the estimate for it, as I am afraid it will already amount to much more than was originally contemplated, that four or five capacious wells (those already existing, if repaired, would answer probably), domed over, should be sunk in the lowest portions of the reservoir, each well connected by a masonry duct, or pipe, with a proper fall; certain portions being built dry, with apertures sufficient to admit of the leakage of the water through, but of so small a size as to exclude the formation of deposit in them. These ducts or pipes, being connected with each other, should communicate with the bottom of the inlet tower, and whether the reservoir silted up or not, they would always afford a constant supply of water.' No evaporation could take place, as they would in time be probably covered with the deposit, this débris holding in suspension vast quantities of water, which would find its way to a lower level into these covered wells, and thence, by the inlet tower, to the conduit pipe or aqueduct. Supposing such to have been built at the period the Upper Kartriz dam was constructed, they would now be the sources of a vast and constant supply from the débris itself. I never heard of wells failing in the bed of a dry tank, nor have I ever known them even running dry when in the vicinity of such a water-bearing medium.
- 51. The total length of the cut will be 7,900 feet, with a general section similar to that of the present Nullah; width at bottom 40 feet, with the banks properly sloped.
- 52. The inlet tower is the means by which the water from the reservoir is filtered, and the supply regulated to the conduit pipe or aqueduct. In tanks

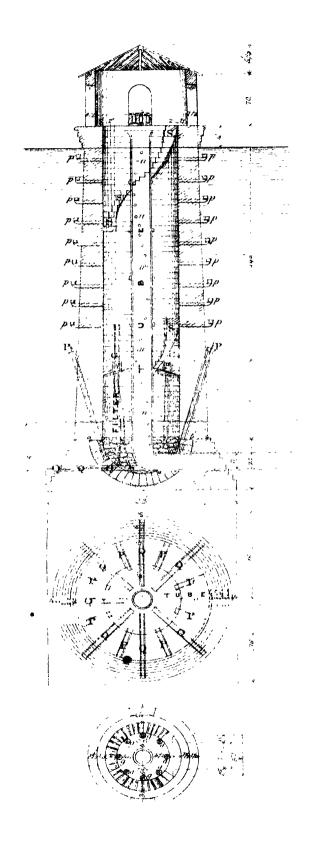
^{*} There is a large well of this sort in the bed of the Lower Kartiz tank, at its western end near the dam, connected with the aqueduct through the dam wall. On this well, and the water stored in different reaches of the aqueduct itself, is the City dependent for the supply in the middle and at the end of the hot weather. Doubtless the deposit, holding vast quantities of water in suspension, is the source from which the well is supplied; and it is not at all improbable that it covers a spring, which was in existence before the dam was built.

in the Deccan formed with masonry dams, the same principle is not unfrequently observed. A square tower is built in front of the Bund wall, with steps down it, or more commonly holes left in the sides to receive the feet when ascending or descending. The water from the tank is let into the tower by orifices in the wall furnished with plugs, and from thence the supply is regulated to the aqueduct. In some of the works in England an inlet tower of the description proposed is used, while in others (in the Manchester Water-works, I think) there is no tower, but a brick shaft communicates with a culvert running through the embankment in which the supply-pipes are laid. Rods pass up this shaft communicating with the valves in the pipes, and the supply is regulated from the top of the dam. The annexed Diagram will show what I mean.

I have, however, for the reason before explained, a strong objection to interfere with the earthen mound, beyond what is absolutely necessary, or cannot be avoided; I have therefore placed the inlet tower at the foot of the embankment, in the deepest part of the reservoir, in order to obtain the water from the reservoir when at its least possible level. The bottom of the pipe, or aqueduct, has been taken nine feet below the bed of the stream.

53. In order better to explain the use and purposes of this work, I attach a Diagram, to which reference is requested in the following account of it. In the foundation of the work, seven cuts (q, q, q, o, o, o, o) are left filled up with dry stone, radiating from the centre of the tower, through which any springs or drainage can enter at the base of the filter, or water percolate through from the reservoir itself. At a height of 15 feet above the footings of the foundations, iron pipes (P, P, P), eight in number, are built into the walls of the tower, passing obliquely through them. They are placed in the intermediate spaces between the cuts above mentioned (q, q, q). The water from the reservoir enters the pipes (P, P, P) and the cuts (Q, Q, Q)at the bottom of the filter. From this it will be seen that the base of the tower is intended to act as a filter; it is 13 feet in diameter and 23 feet in depth. At the point where the mouths of the pipes (P, P, P) are built into the tower, that is, 15 feet from the footings, the filter is domed over with a stone floor, in which the iron gratings (q, q, q) are built, and screwed down to the stone floor. The water therefore can only enter the tower at the bottom of it, and passes





through the filter by the gratings into the body of it, to obtain the same level as the water in the reservoir outside the tower.

In the interior of the tower, between the top and the domed stone floor (G, G), a staircase of stone (S, S) runs spirally round the inside of the tower. Down the centre of it, extending from the top to the base, an iron cylinder or tube runs, in direct communication with the pipe or aqueduct which it joins. This tube is altogether independent of the tower; it rests on the base of it, and may probably require ties in one or two places to keep it steady, passing through the outer walls of the tower; it passes through the floor (G) to the very base of the tower. This tube is provided with tubular orifices and plugs at every two feet of vertical height, running spirally round it, corresponding with the steps, or the man descending would not be able to reach the plugs for the purpose of opening and closing them. The water then from the reservoir enters the very bottom of the tower, ascending through the filter into the upper part, to the same level as the water outside, and is let off into the tube through the tubular orifices, by opening or closing the plugs (u, u, u, u, u). The tube is proposed to be three feet in diameter, more than its actual requirements as regards the water passing down it, to enable a man or boy to be let down at any time to refix or overhaul the My object in the adoption of the tube was the avoidance of all sluices, the management of which natives do not comprehend and cannot repair. Plugs answer the present object equally well, and, what is better still, any native understands their object and use. These plugs (u, u, u, u) have been carried to the very bottom of the filter almost, to take advantage, in seasons of peculiar drought, of every drop of available water. In cases of emergency, I have also provided for tubular orifices (p, p, p, p), to communicate direct with the reservoir, and through which unfiltered water can be let into the tube when required. The discharge through these tubular orifices (p, p, or u, u) would be, at depths of 2, 4, and 6 feet respectively, 1.80, 2.55, and 3.12 cubic feet of water per second. Appendix, lettered D.

54. There may be some objection raised, and with reason too, of the difficulty of clearing out and renewing such a filter; it will, however, last some years; and in seasons of drought, at the close of the hot weather, when the water in the tank is low, the sand might

be partially removed and renewed. Supposing even that it does not completely answer its purpose, of which I have no doubt,—it is the difficulty of renewing it which I consider objectionable,—but little extra cost has been incurred; it is, after all, only the filling in with sand, &c. The construction of a filter is a work that can be executed at any time hereafter; and supposing the masonry aqueduct to be adopted, I doubt whether the attrition of the water against its sides will not be sufficient, and that a filter will be required at all; at any rate, it is a portion of the work which I do not feel inclined, at present, to incur any expense in undertaking.

- 55. The gangway is the passage across from the embankment to the inlet tower. It is formed of two trusses, 5, - Gangway. on Horne's patent principle; one of the simplest description of wooden bridge yet invented, and at present much adopted, both in Canada West and in the United States, for road and railroad bridges. The total length of the gangway is 180 feet, with a clear breadth of passage of $5\frac{1}{4}$ feet, supported at one end by the inlet tower, at the other by a pier built in the embrukment, and in the centre by a high pier; this gives two clear spaces of 844 feet each, the pier in the centre being 4 feet thick at top. The trusses are composed of top and bottom chords, with diagonal bracing between the Between the upper and lower timbers of each truss there are main and counter braces, abutting on cast-iron brace-blocks, with iron suspension rods between. The planking is 35 inches thick, laid across the two trusses; there are no joists.
- 56. In order to compare the cost of conveying the water from the main to the distribution reservoir in the Camp, Plan No. 8 contains longitudinal sections for both an iron pipe and masonry aqueduct. The total length of the former is 27,343 feet = 5 m. 1 fur. 94\frac{1}{3} yds.; of the latter, 41,116 feet = 7 m. 6 fur. 65\frac{1}{3} yds., the pipe

943 yds.; of the latter, 41,116 feet = 7 m. 6 fur. 653 yds., the pipe being shorter than the aqueduct by 2 m. 4 fur. 191 yds., the measurements commencing from the starting point of either, the centre of the inlet tower, and ending at the Camp distribution reservoir, marked A in Plans Nos. 1 and 8. The course of both is the same for a distance of 11,478 feet = 2 m. 1 fur. 86 yds., from the main reservoir to the middle of the Duncowree Valley; the iron pipe from this point taking an almost direct course towards the Camp, while the aqueduct neces-

sarily follows the sinuosities of the hills, to avoid the heavy work which a more direct course would entail, and which does not in the same manner affect the line of the pipe.

57. I will now describe the course of each of these modes of con-

Description of the course of the iron pipe.

veyance separately, commencing with the iron pipe. As before observed, the pipe and aqueduct maintain the same line for a dis-

tance of 11,478 feet = 2 m. 1 fur. 86 yds.; from the centre of the inlet tower, 9 feet below the bed of the Nullah, it runs with a slope of a quarter of an inch to 100 feet, for a length of 4,729½ feet, to the head of the little plateau dividing the Kartriz and Ambeygaum valleys. At this point the tunnel commences, along which the pipe is laid, from the bottom of a masonry shaft 4 feet square, sunk 61 feet below the surface of the ground. From this last shaft the pipe is laid for a distance of 2,570 feet, at an inclination of 0.84 foot to 100 feet, to a chambered shaft, constructed over a very promising spring (No. 3, Plan No. 8), and sunk 21 feet below the surface of the ground. This shaft is divided into two compartments, the water from the pipe being discharged into one, and let off into the other by means of plugs in the division wall. From the base of the shaft No. 3 the pipe leaves, for a distance of 3,255\frac{1}{2} feet, at a slope of 1.59 to 100 feet; and for another 594 feet, at a slope of 3.72 to 100 feet; it then rises and falls at various inclinations noted on the Plan for the remainder of its length, a distance of 13,422.5 feet, until it reaches the Camp distribution reservoir. The head of water from the bottom of the shaft No. 3 (in Plan No. 8) is 31.5 feet, which is considered ample to overcome the different rises in its length. There is no necessity, in the whole length of the pipe, for any aqueducts or other works, beyond the mere trench in which it is laid.

58. The masonry aqueduct leaves the centre of the inlet tower at

9 feet below the bed of the Nullah, being connected with the iron tube by means of a
masonry chamber, and runs with an inclina-

tion of a quarter of an inch to 100 feet for a distance of $4,729\frac{1}{2}$ feet, to the entrance of the tunnel, which leaves the bottom of a masonry shaft 4 feet square, sunk 61 feet below the surface. It then proceeds along the tunnel level, for a distance of $2,781\frac{1}{2}$ feet, to a similar masonry shaft, sunk $65\frac{3}{4}$ feet, at the head of the Duncowree Valley.

On quitting the tunnel it proceeds with an inclination of 0.84 foot to 100 feet for a distance of 2,570 feet. This portion is divided into two falls by two chambered shafts (Plan No. 8, Nos. 3 and 4), respectively 21 and 31 feet deep below the surface. The water is received into one compartment of the shaft, and discharged into the other by means of plugs in the division wall (see details in Plan No. 7). No. 3 shaft is that covering the spring, allusion to which was before made in the description of the line of iron pipe. It then again falls with an inclination of 1.81 feet to 100, for a further length of 1,397 feet, the distance being divided into two falls by chambered shafts Nos. 1 and 2 (Plan 8, details Plan 7), sunk respectively 11 and 19% feet below the surface of the ground. From the last shaft (No. 1) to the camp distribution reservoir is a distance of 29,638 feet, over which the aqueduct is carried at a uniform slope of a quarter of an inch to 100 feet, being provided with air-shafts at every 500 feet. Throughout this last portion of its length, the only work which will be required is. raising it about 6 feet high to a length of above 1,200 feet, over a hollow portion of ground not far from the Camp distribution reservoir.

59. The section of the aqueduct is the same throughout, except for that portion between shafts Nos. I and 5, a distance of 3,967 feet, which, owing to the unavoidable slope, it has been considered advisable to make of a longer and stronger section; and for the portion passing under the embankment, the head is arched instead of flat. The tunnel along which the water runs, being a distance of 2,781½ feet, is also of a different section. The details of all which are plainly shown in Plan No. 7.

- 60. The Appendix, lettered D, contains four different calculations for finding the diameter of the pipe.

 for finding the diameter of the pipe, the least being calculated at 10.64, and the greatest at 13 inches. The mean of the whole is 11.5 inches, and allowing 1-6th of the diameter so calculated, to be increased to meet loss of head, apart from that of friction, which I believe to be usual in practice, would make the diameter of the pipe 13.41 inches. Thirteen inches would, I therefore think, be a safe size to allow for.
- * 61. Under the letter above referred to in the Appendix will also be found five different formulæ, with the answers attached, for the purpose of ascer-

taining the area of the aqueduct. The least side is 9.94 inches, and the greatest 13.32 inches. The mean of the five is 11 inches, or 121 square inches. I have therefore given the aqueduct an area of 270 inches, a little more than double that of the calculations, being 15 inches high by 18 inches broad, interior measurement. This, I think, will be found necessary; and were it not for the increased outlay, I should feel much inclined to make it of such a height and breadth that a boy could enter and be able to clean it out when required. This however, would considerably enhance the first outlay; constructed of the present size, when requiring to be cleaned, it must be opened out in places.

62. The water from the pipe or masonry aqueduct, as generally 7.—Distribution Reservoirs.

Two and one day's supply.

described in paragraph 34, will be led to a distribution reservoir, proposed to be excavated in a vacant piece of ground in front of the solitary cells, the difference of level between which and the bed of the Ambeygaum Nullah, where the inlet tower is to be constructed, is 80.25 feet. This reservoir will contain either one or two days' supply. Plans 6 and 7 show the details of both reservoirs. The point marked A on Plans Nos. 1 and 8, or 8 on Plan No. 9, is almost the highest point in Cantonment, being 224.5 feet above the top of

the Jamsetjee Bund; so that, from a reservoir situated on this ground, there is not an inhabited portion of the Camp which cannot be supplied with water by gravitation, besides giving a considerable head for the flushing of sewers and drains.

63. From paragraph 37 it will be seen, that one day's supply is 821,917 gallons, or 131,929 cubic feet, calculating for the supply of the reservoir in round numbers at three hundred million gallons; but, taking forty thousand as the population at twenty gallons each per day, for two days the contents of such a work would amount to 256,821 cubic feet, or for one day to 128,410 cubic feet; and, if storage room be afforded for these quantities, it will, I think, be quite sufficient. By referring to Appendix, lettered E. the dimensions of a distributing reservoir for a two days' supply will be 201 long × 130 broad × 7 deep; and for one day's supply 170 long × 130 broad × 7 deep. The larger reservoir I have divided into two parts by a central wall, in order that one may be cleaned out while the other is in use, and as always furnishing a sufficient supply for flushing sewers

and drains. In the smaller reservoir there is no division wall. the larger the orifices and plugs which supply the two halves are quite distinct, so that either of the halves can be filled, while the other is being cleaned out. The general plan of both reservoirs is rectangular, with a high wall surrounding them. The water from the supply-pipe, or masonry aqueduct, is conveyed into the cistern C (Plans 6 and 7), and from the cistern is again discharged into the From this reservoir it is received into another reservoir (N) on the opposite side, from which last the distribution pipes of the Camp are furnished. In the line of the steps of both cisterns, so as to be easily accessible, orifices are made in the cistern walls at one foot central distance apart, with plugs on both sides. These orifices are 6 inches in diameter, and the discharge per second, at depths of 2, 4, and 6 feet, will be 1.38, 1.95, and 2.39 cubic feet respectively. (See Appendix, lettered D.)

- 64. From the distribution reservoir above described, a series of iron pipes will convey the water to the different 8.—Camp Distribution. parts of the Cantonment, with an iron standpipe and stop-cock at each eistern; the pipes being always charged The cisterns are so placed as to be most conveniently situated for the neighbourhood they are intended to supply; but for each hospital (except the Staff hospital, which is between the cistern and the N. I. Regiment Hospital, and Sapper and Miner Lines) one of the smaller description of cisterns 15 broad × 10 long × 8 deep, containing 1,200 gallons, is placed in the compound. There are three principal branches of pipes, one for supplying the Horse Artillery Lines, one for the New Wanowree Barracks, Native Infantry Regiments, and Ghoreporce Barracks, and another for the supply of the Bazar, Commissariat, Sappers and Miners, and Officers' Lines. A branch also runs to the Malcolm Tank in the Bazar, which is the only reservoir in a fit state ready at hand, that I have been able to avail myself of, to fill from the distribution reservoir.
- 65. By a reference to the Appendix, lettered F, it will be seen that the numbers and capacities of the cisterns have been apportioned, as nearly as practicable, to the wants of each locality. In all, thirty-six cisterns will be required, twenty-four of the larger sort, $25 \times 18 \times 8$ feet, and of the smaller twelve, $15 \times 10 \times 8$ feet, each containing 2,000 and 1,200 cubic feet respectively. The capa-

cities of these cisterns, with that of the Malcolm Tank, give 1,21,666 cubic feet, which is sufficiently near to the quantity required for all practical purposes. The cisterns are supposed to be filled daily from the hydrant which is attached to each, or oftener, if necessary, by one of a regular establishment in whose charge the works will remain.

- 66. The total length of the pipes required for the Camp distribution is entered in the Appendix, lettered G, and amounts to 8 miles, 1,643 yards, 2 feet, the first half of which should be 5-inch, and the latter half 4-inch pipes. Plan No. 9 shows the proposed distribution in detail, in which are entered the various heights of the different localities above the Jamsetjee Bund.
- 67. The amounts of the estimates of the various works for supplying the Camp of Poona with water, from the Ambeygaum Valley, are as follows:—

•	Rupees.
The embankment	1,52,064
• The waste weir	10,517
The artificial cut to carry off the first floods	24,651
The inlet tower	12,172
The gangway	9,082
The masonry aqueduct and tunnel	1,14,969
The 13-inch iron conduit pipe and tunnel	1,85,434
The distribution reservoir, to contain a two days'	
supply	36,834
The distribution reservoir, to contain one day's	
supply	22,599
The Camp distribution	1,03,409

Of which I would beg to recommend the following for adoption, as follows:—

	Rupees.
The embankment	1,52,064
The waste weir	10,517
The artificial cut to carry off the first floods	24,651
The inlet tower	12,172
The gangway	9,082
The masonry aqueduct and tunnel	

	Rupees.
The distribution reservoir, to contain one day's	
supply	$22,\!599$
The Camp distribution :	
Total	4,49,463

Amounting to Rupces four lacs, forty-nine thousand, four hundred and sixty-three.

68. The Establishment, as per margin, was placed at my disposal

Surveyor and Builder, Venaik Bhickajee.

Sub-Assistant Surveyor and Builder, J. Wainwright.

Probationer Sub-Assistant Surveyor and Builder, Muncherjee Cowasjee. by the Chief Engineer of Public Works for the surveys and sections required for this project, and I have much pleasure in acknowleaging the valuable assistance which I have received from Surveyor and Builder, Venaik Bhickajee.

> (Signed) PHILIP L. HART, Captain Engineers, on Special Duty.

Poona, 23rd October, 1857.

APPENDIX.

A.

No. 515 of 1857.

From the Acting Deputy Assist. Quarter Master General, P.D.A. To Captain P. L. Hart, Engineers, on Special Duty, Poona.

Acting Deputy Assist. Quarter Muster General's Office, P.D.A.

Poona, 24th February, 1857.

Sir,—In acknowledging the receipt of your letters Nos. 12 and 19, dated the 17th and 23rd instant respectively, I have the honour to annex, for your information, an estimate of the whole population of the Cantonment of Poona, supposing it to be fully garrisoned, as follows:—

	Num	ber of
CORPS AND DEPARTMENTS.	Population.	Cattle.
Artillery, two Troops of Horse Artillery, and European Company of Artillery with Light Field Battery	4,674	680
Head Quarters Sappers and Miners; three Companies.	1,363	64
Two European Infantry Regiments Two Native Infantry Regiments	4,880 4,634	298 278
Ordnance Department ,	515 17,828	32 1,686
Bazar, Commissariat Department, Pensioners, &c. Staff; civilians and others unconnected with Regi-	,	Í
ments, non-residents	600	150
$\operatorname{Total}\ldots$	34,494	3,188
Population 3	34,494	
Z 1	2 100	

Total.... 37,682

2. Great care has been taken in framing this Return, and it is believed it will prove correct in all particulars.

I have, &c.,

(Signed) J. C. Coley, Captain, Acting Deputy Assist. Quarter Master General, P.D.A. (True copy)

PHILIP L. HART, Captain Engineers, on Special Duty.

Fall of Rain in Poona, for the last Twelve Years, extending from the Year 1845 to the Year 1856, gauged at the Staff and Civil Hospitals.

1856.	Cents		128
	ē.	:::::::::::::::::::::::::::::::::::::	19
1855.	Cents		543
	li ii		33
1854.	Cents	55. 55. 57. 59. 59. 59. 59. 59.	$10\frac{1}{2}$
	li.	· · · · · · · · · · · · · · · · · · ·	3.4
1853.	Cents	: : : : : : : : : : : : : : : : : : :	98
	Ę	:::::::::::::::::::::::::::::::::::::::	. 52
1852.	Cents	.: 34 18 19 19 10 10 10 10 10 10 10 10 10 10 10 10 10	SS
Ä	Ē		3
851.	Cents	55 188 188 198 198 198 198 198 198	61
=	Ë	:::: : 	21
1850.	Cents	15. 15. 15. 15. 15. 15. 15. 15. 15. 15.	+11
132	Ë	· · · · · · · · · · · · · · · · · · ·	51
1849.	Cents	28. 	30,
1 22	In.	:::::::::::::::::::::::::::::::::::::::	33
848.	Cents		3
2	1 3 1		15
1847.	Cents		$73\frac{1}{2}$
2	In.		0.7
1846.	Cents		342
18	Ę		55
1845.	Cents	::: :::::::::::::::::::::::::::::::::::	χ. Σ
22	ġ		Ξ.
·		January February March April May June July August September October November	Total
		January February March April May June July September October December	

(True Copy)
(Signed) PHILIP L. HART,
Captain Engineers, on Special Duty.

Staff Surgeon. (Signed) T. B. LARRINS,

(Signed) J. Kettu, Civil Surgeon.

Abstract of Twelve Years' Rain-fall at Poona, from 1845 to 1856.

		14 25 20 15	78 34 $\frac{1}{5}$ 73 $\frac{1}{2}$ 52
• •	1	I I	52°
• •		33 19	$\frac{78}{47\frac{1}{2}}$
		32 37	$19 \\ 38 \\ 98 \\ 10 \frac{1}{3}$
• •	• •	34 19	54½ 87
	 Total.		22 32 37 34 34 19

Least rain-fall was in 1845, amounting to	The average rain-fall of twelve years is	.25.89
The greatest rain-fall was in 1853, amounting to	Least rain-fall was in 1845, amounting to	.14.78
The grounds runt was in 1995, unfortung to 1997.	The greatest rain-fall was in 1853, amounting to	$.37 \cdot 98$

(Signed) Philip L. Hart, Captain Engineers, on Special Duty.

Return of the Water gauged at the Great Konducah Nallah, from the 24th February to the 10th June, 1857.

	Remarks.	From the 24th to the 26th February, 1857, the Nullah was gauged a first below Captain Graham's	16.2.) Gays after the Wilsh dated up at 16.2.! has spot, and on the 7th March 13.10.5; the gauging was recommenced class to the yillage of Given Kond-	Average 1,611's gal- from and one of the most prolific lons per hour.	pious spangs. The underground dialnage finding its way into this stream is very considerable. There are no less than seventeen Pawn	gardens structed on its banks, watered by twenty wells, sunk, of course, considerably below its bed, but the water from which feeds than	
June, 1857.	Date. feet per pour.	Nulla d 270 270	250 250 250 216	Average 1,61 f's gallons per hour.			
May, 1857.	Date for per lour.	έi	9th 135 × 11-05 5th 14th 270 1682-1 6th 16th 216 1345-68 9th 18th 216 1345-68	7777	nlla –	28th 270 1682 ⁴ 29th/860 2242 ⁸ 80th/270 1682 ⁴	Average 1,367.2 gallons per hour.
April, 1877.	Cubic Gallers Date, for yer hour, hour.	2nd 270 16 2 21 6th 270 16×21 8th 216 1513 68	10th 216 1345-68 14th 180 1121-4 15th 180 1121-4 16th 180 1121-4	20 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	150 1131.4	Average 1,252.2 gal-25th 270 lons per hour. 30th 270	
March, 1857.	Cubic Gallons Date, feet per per hour.	7th 270 12th 270 14th 270	1682-1 1682-1 56 11922-32 11495-2	11652.1	lons per hour.		
. February, 1857.	Oubic Gallons Date. feet per per hour.	24th 18·62 116 26th 14·59 90·89	Average 103.4 gal. 18th 270 lons per hour. 21st 308.				

Camp Poona, October 23rd, 1857.

(Signed) PHILIP L. HART, Captain Engineers, on Special Duty.

Return of the Water gauged at the back of the Upper Kartriz Tank, from the 23rd February to the 9th June, 1857.

		1											-	-	
Fe	February, 1857.	1857.	A	March, 1857.	57.	¥	April, 1857.	7.	4	May, 1857.	7.		June, 1857.	57.	
Date.	Date. feet per hour.	Cubic Gallons eet per hour.		Cubic Gallon- Date, feet per per hour, hour.	Gallons per hour.	Date.	Cubic feet per pour	Gallons per hour.	Date.	Cubic feet per hour.	Gallons per hour.	Date.	Date.: feet per per hour.	Gallons per hour.	Remarks.
23rd 25th 28th	54.0 72.0 72.0	336.4 448.5 418.5	4th 6th 9th 12th 13th	4th 77·14 6th 77·14 9th 77·14 12th 77·14 13th 54·0	480.5 480.5 480.5 480.5 336.4				1st 6th 8th 14th 16th	20.76 18.0 16.87 90.0	129-3 112-1 105-1 560-7 560-7	2nd 4th 5th 6th	45.0 45.0 56.0 33.75	280.3 280.3 280.3 210.5	The point where these measurements were taken was close behind the Bund wail of the upper tank, so as to embrace the water issuing from the tank itself, the leaking through it, and the water escaping from a spring behind the eastern end of the Bund wall. The
Ave. lo	rage 41 ns per	Average 411'1 gallons per hour.	19th 19th 20th	16th 45·0 17th 43·2 19th 43·2 20th 41·53	258.1 258.1 258.1 258.1 258.1	2) st 2) st 2) st 2) st 2) st 2) st	21 C1	1.01	20th	######################################	20.5 20.5 20.5 20.5 20.5 20.5 20.5 20.5	Avere	Average 255.1 gallons per hour.		think glandarily utteut up to the 10th May, the water very rapidly decreasing, until a space was left in the deepest part of it round the steps of about \$28,000 square feet. On the 13th, \$6th, \$27th May heavy showers fell, which
			26th 30th	54.0 45.5	336.4 250.3		20.76	129.3	15 th	77.14 77.14 90.0	480.5 480.5 560.7				increased the supply. At the end of July the tank was almost dry. From a careful observation of this effete work. I am of opinion the tank
			Aver	Average 357.4 gallons per hour.	·4 gal-	Avera	Average 171.4 gal- 28th lons per hour. 30th	·4 gal- our.	29th 29th 30th	90.0 67.5 54.5	560·7 420·5 336·4				in its present state as a supply is not to be depended on. No doubt by tapping the deposit with which it is perhaps two-thirds filled, a considerable quantity of water would be obtained. I
									Avera lon	verage 419·4 ge lous per hour.	Average 419.4 gallous per hour.				should say the evaporation in this ex- tensive shallow expanse was in all possibility very considerable.

(Signed) PHILIP L. HART, Captain Engineers, on Special Duty.

Camp Poona, October 23rd, 1857.

Return of the Water gauged at the Large Nullah of the Ambeygaum Valley from the 24th February to the 8th June. 1857.

		Remarks.	Average 1,121-4 gallons per hour.
	June, 1857.	Date. feet per per hour.	2nd Kutcha dam washed away. 3rd Rebuilding it. 4th 180°0 1121-4 6th 180°0 1121-4 8th 180°0 1121-4 gallons per hour.
oth June, 1804.	May, 1857.	Date. feet per per hour.	1223.3 3t 120.0 747.6 51223.3 6th 120.0 747.6 1121.4 8th 108.0 672.8 672.8 672.1
ww.	April, 1857.	Date. feet per per I hour.	1st 196°36 1223°3 1st 120 3rd 196°36 1223°3 6th 120 3rd 180°0 1121°4 8th 108 5th 180°0 1121°4 13th Nasi 5th 185°0 841°0 14th Resident 180°0 1121°4 180°0 180
	March, 1857.	Cubic Gallons Date. feet per por hour.	4th 308-56 1922-3 7th 308-56 1922-3 12th 270-0 1682-1 16th 270-0 1682-1 19th 308-56 1922-3 21st 216-0 1345-6 25th 196-36 1223-3 25th 216-0 1345-6 Average 1,630-7 gallons per hour.
	February, 1857.	Cubic Gallons Date. feet per per lour.	24th 360.0 2242.8 25th 308.561922.3 27th 308.561922.3 Average 2,029.1 gallons per hour.

Camp Poona, October 23rd, 1857.

(Signed) PHILIP L. HART,
Captain Engineers, on Special Duty.

General Result of the Gauging of these three Sites.

General Remarks.		From these results it will be seen that the supply of the Great Kondwah Valley is the greatest, which coin ides with the opinion formed of it from frequent personal observation. This valley is in general character that the opinion formed of it from frequent personal soil resting on a sort of white mat. The flat and open, and the Led in the lower portions formed of an alluvial soil resting on a sort of white mat. The running at the bottom of it. What I intend to observe is, that the water percolates through the soil, not rapidly running off it. Large quantities of water are thus held in suspension, which however show but little on the surface, but which an excavation of moderate depth quickly discovers. There are consequently various springs on its banks, and twenty wells working to supply seventeen Pawn gardens. An excavation to a few feet below the bed of the Nullah on its banks soon reaches water. With respect to the storage capacity of the Upper Kartriz Tank, it cannot, I think, be depended on for a full.	Average Average and constant supply of ward: The stationaries of disposit, with which I should say, be very rapid. There is doubtless a large supply held in suspension by the Average region much! I should say the tank was almost 2-34ds filled up. This has only to be tapped, and led out at 300 gal. 1,179.4 the back of the Burd, but what the supply would be, thus obtained, it is almost unpossible to conjecture. Source gallous The supply of ward to the Ambogramun Valloy from these gaugings, though less than that of the Great bour. The valley is surrounded by hills on hour. Source for the Ambogramun Valloy from the supply of a large reservoir. The valley is surrounded by hills on hour lall safe, and, as compared with the latter, has little alluvial soil in its bed, the soil being generally of moorum with a few feet of reddish carch above, except in the lower part of it meat the village, where it is proposed to writh a few feet of reddish carch above, except in the lower part of it meat the village, where it is proposed to	Note.—The two days in February have been the water rapidly riving, weeping down like a torient, and soon running off. There are not more than two or nitted, as the period is too short to take an three wells on its banks. With respect to its capabilities for storage and as a catchment basin, it possesses, I terage of to be of any use.
Ambey- aum \ul- lah.	allons per hour.	1,430·7 931·4 1,031·4 1,121·4	Average 1,179-4 gallons per hour	ry have beer t to take at
Back of the Amber-Upper Kar-gaum Vultriz Tank.	Gallons per C hour.	357.4 171.4 419.4 255.1	Average 300 gal- lons per hour.	s in Februa is too shor use.
Burra Bs Kondwah U Nullah. t	Gallons per Gallons per Gallons per hour.	1,688·7 1,252·2 1,367·2 1,614·8	Average 1,480.7 gallons per hour.	Note.—The two days omitted, as the period average of to be of any
Month and Year.	1857.	March April May		Note.—' omitted, a average où

(Signed) Purlir L. Harr, Captain Engineers, on Special Duty.

Camp Poona, 23rd October, 1857.

D.

Calculations to determine the Diameter of the Conduit Pipe, the Masonry Aqueduct, the Discharge through the holes in the Iron Tube of the Inlet Tower, and through the orifices in the sides of the Distributing Reservoir.

THE CONDUIT PIPE.

Total length of the pipe is 27,343 feet = 5 m. 1 fur. $94\frac{1}{3}$ yds. Total fall is 80 feet.

The discharge is 1.52 cubic feet per second.

Inches.

1. By Hawksley's formula, the diameter will be 11·20
2. By Beardmire's ditto, ditto 10·71
3. By De Prony's ditto, ditto 13·46

4. By De Buat's ditto, ditto 10.64

$$(1) d = \frac{1}{16} \sqrt{\frac{9? l}{h}}$$

(2)
$$x \qquad 0.235 \sqrt[5]{\frac{9^2 \times l}{h}}$$

(3)
$$d = \sqrt[5]{\frac{4 \cdot 9^3}{(53 \cdot 58 \times 7854)^2}} J$$

THE MASONRY AQUEDUCT.

Total length of the aqueduct is 41,116 feet = 7 m. 6 fur. $65\frac{1}{3} \text{ yds}$. Total fall is 80 feet.

Discharge 1.52 cubic feet per second.

1. By De Buat's formula, the side of the aqueduct is. 9.96
2. By De Prony's formula, ditto . 9.94

- 5. By Tardini's formula, the side of the aqueduct is . 13.32

(1)
$$v = \frac{307 (\sqrt{d-0.1})}{\sqrt{s-h l \sqrt{s+1.6}}} - 0.3 (\sqrt{d-0.1})$$

(2)
$$\frac{S^3}{P} = \frac{1}{\phi} (a \ Q \ S + b \ Q^2)$$

$$(4) s\sqrt{\frac{8}{C}} = \sqrt{\frac{Q^2 \times \frac{1}{\phi}}{2736}}$$

$$(5) lh \sqrt{h} = Q \sqrt{\frac{1}{\phi}}$$

DISCHARGE THROUGH THE HOLES IN THE IRON TUBE OF THE INLET TOWER.

 $Q = 5.1086 d^2 T \checkmark H.$

Cubic feet.

The discharge per second at the depth of 2 feet is . . 1.80

Ditto ditto 4 ,, . . 2.55

Ditto ditto 6 , . . . 3.12

DISCHARGE THROUGH THE ORIFICES IN THE SIDES OF THE DISTRIBUTING RESERVOIR.

$$Q = 4.978 A T \checkmark II.$$

Cubic feet.

The discharge per second at the depth of 2 feet is . . 1·38

Ditto ditto 4 ,, . . 1·95

Ditto ditto · 6 ., . . 2·39

(Signed) Philip L. Hart, Captain Engineers, on Special Duty.

Poona, 23rd October, 1857.

E.

Calculations of the Contents of the Distributing Reservoir, to hold One or Two Days' Supply.

RESERVOIR TO HOLD ONE DAY'S SUPPLY.

Pop. Day Gall. $\frac{40,000 \times 2 \times 20}{6023} = 256,821$ cubic feet = two days' supply.
$\frac{693}{256,821}$ = 128,410 cubic feet = one day's supply.
Cultin foot
Reservoir $201 \times 110 \times 7 = \dots 154,770$
Deduct,—Two distributing cisterns,
$2 \times 26 \times 13 \times 7 = 4,732$
,, One foot above top water,
$201 \times 110 \times 1 = 22{,}110$
26,842
Cubic feet
Contents required128,410
Less than requirement by
RESERVOIR TO HOLD TWO DAYS' SUPPLY.
Two days' supply as above, cubic feet 256,821.
Ft. long. ft. broad. ft. deep. Cubic feet.
Reservoir $270 \times 165 \times 7 = \dots 311,850$
Deduct,—Two distributing cisterns as above 4,732
,, Division wall 165 n° 2×13
$= 139 \times 7 \times 2\frac{1}{2} \dots \qquad 2{,}432$
,, One foot above top water,
$270 \times 165 \times 1 = \ldots 44,550$
51,714
Cubic feet
Contents required
In excess of requirement 3,315
(Signed) PHILIP L. HART, Captain Engineers, on Special Duty.
Poona, October 23rd, 1857.

Poona, October 23rd, 1857.

F.

Calculations for the Cisterns of the Camp Distribution.

Popu-lation.

Three Companies of Sappers and Miners 1,363 64 1,427 28,540 4,581.5 One European Infantry Regiment
pean Company of Artillery, with Light Field Battery
Field Battery 1,674 680 5,354 107,080 17,187.8 Three Companies of Sappers and Miners 1,363 64 1,427 28,540 4,581.5 One European Infantry Regiment 2,440 149 2,589 51,780 8,311.3 Dutto ditto 2,140 149 2,589 51,780 8,311.3
Three Companies of Sappers and Miners . 1,363 64 1,427 28,540 4,581 5 One European Infantry Regiment
Ditto ditto ditto 2,410 149, 2,589 51,780 8,311:3
Ditto ditto ditto 2,440 149, 2,589 51,780 8,311·3
One Native Infantry Regiment 2,317 139 2,456 49,120 7,884.4
Ditto ditto ditto 2,347 139 2,456 49,120 7,884 4
Ordnance Department
Bazar, Commissariat, Pensioners, &c 17,825 1,686 19,514 390,280 62,645 2
Staff, Civilians, &c

Total.... 34, 194 3188, 37,682 753,640 120,969.6

Require- | No of

Cattle. Total.

Gallons Cubic feet per day. per day.

Population $37,682 \times 20$ gallons daily = 753,640.

Number of Cisterns required.

	Requirements in Cubic Feet per Day.	No. of Cis- terns.	Cubical Size of Cisterns, Contents of Cisterns,
Two Troops of Horse Artillery, and European Company of Artillery, with Light Field Battery	17,187	$\left\{\begin{array}{c} 4\\1\end{array}\right.$	$\left\{ \begin{array}{c} 25 \times 18 \times 8 \\ 15 \times 10 \times 8 \end{array} \right\} \left\{ \begin{array}{c} 14,400 \\ 1,200 \end{array} \right.$
Three Companies of Sappers and Miners	4,581	$\left\{ \begin{array}{c} 1\\1 \end{array} \right]$	$\left\{\begin{array}{c c}25\times18\times8\\15\times10\times8\end{array}\right\} \left.\begin{array}{c}3,600\\1,200\end{array}\right.$
One European Infantry Regiment	8,311	$\begin{cases} 2\\1 \end{cases}$	$ \left\{\begin{array}{c} 25 \times 18 \times 8 \\ 15 \times 10 \times 8 \end{array}\right\} \left\{\begin{array}{c} 7,200 \\ 1,200 \end{array}\right\} $
Ditto ditto ditto	8,311	$\left\{\begin{array}{c} 2\\1\end{array}\right\}$	$ \left\{\begin{array}{cc} 25 & 18 \times 8 \\ 3 \times 10 \times 8 \end{array}\right\} \left\{\begin{array}{c} 7,200 \\ 1,200 \end{array}\right\} $
One Native Infantry Regiment	7,884	} 2	$ \left\{\begin{array}{c} 25 \times 18 \times 8 \\ 15 \times 10 \times 8 \end{array}\right\} \begin{array}{c} 7,200 \\ 1,200 \end{array} $
Ditto ditto ditto	7,881	$\begin{bmatrix} 2 \\ 1 \end{bmatrix}$	$ \left\{ \begin{array}{c} 25 \times 18 \times 8 \\ 15 \times 10 \times 8 \end{array} \right\} \left. \begin{array}{c} 7,200 \\ 1,200 \end{array} \right. $
Ordnance Department	1,756	(11	$15 \times 10 \times 8$ 2,400
Bazar, Commissariat, Pensioners, &c	62,645	$\begin{cases} \frac{11}{2} \\ \frac{1}{2} \end{cases}$	$\begin{cases} 25 \times 18 \times 8 & 39,600 \\ 15 \times 10 \times 8 & 2,100 \\ 100,0000 & 0.0000 \end{cases}$
Staff, Civilians, &c	2,107	2 2	$\begin{bmatrix} \text{Malcolin Tanks.} & 20,866 \\ 15 \times 10 \times 8 & 2,400 \end{bmatrix}$
Total	120,966	36	121,666

In all, cisterns,	thirty-six	in	number,	of	the	following	sizes,	will	be
required :									

Cubic feet
86,400
14,400
20,886
-
121,666
120,966
5 00
700

(Signed) Philip L. Hart, Captain Engineers, on Special Duty.

Poona, October 23rd, 1857.

G.

List of the different Lengths of Piping which will be required for the Camp Distribution, from the Distributing Reservoir to the various Cisterns, as actually chained on the ground.

From Distribution Reservoir to Hospital old Euro-	Fect.
pean Barracks	350
From the above to the proposed new Artillery Hos-	
pital From the above to the proposed new Artillery	972
Patcherry	900
From the last to the centre of one of the sides Horse Artillery Horse Lines	1,500

* Calculation of the contents of the Malcolm Tanks:-

Depth.		L	engtl.	1.	Wi	ltlı.		Cubic feet.
4.583	×	63	660	i ×	29	.75	==	8,680.493
4.583	×	63	.708	3 ×	29	.83	3 ==	8,710.453
5.833	X	10	×	No.	2	×	29.75 ==	3,470.635
					T	otal	l	20,861.581

By another calculation... 20,866:0

	Feet.
From the last to the front of Horse Artillery Offi-	
cers' Lines	1,180
From proposed new Artillery Patcherry to Horse	
Artillery Barracks	870
From Hospital old European Barracks to centre of	
new Wanowree Barracks	1,400
From the last to the front of new Patcherry	900
From the last to Native Infantry Lines	3,200
From Native Infantry Lines to Native Infantry	
Hospital	700
From Native Infantry Lines to centre of Native	
Infantry Lines	900
From the last to the flank of Native Infantry	
Lines	1,047
From the last to the Tent Lascars' Lines	1,200
From the last to the Hospital Ghoreporee Barracks.	2,700
Branch to the front and rear of the Ghoreporee	
Barracks 1000 + 800	1,800
From Distribution Reservoir to opposite Compound	
No. 90	1,300
From the last to the Solitary Cells	2,360
From the last to the Camel Lines $1,930 \pm 760$	2,690
From the Solitary Cells to the Commissariat Com-	
pound	1,400
From the last to corner of Compound No. 54	968
From last branch to near the old burying ground	1,100
From the last to the Malcolm Tanks	1,374
A branch from the above branch 400 + 650	1,050
From Compound No. 54 to Compound No. 31	1,460
From ditto to Bazar Guard	1,200
From Bazar Guard to West Street	7 90
From Compound No. 31 to Arsenal Compound	1,190
Branch from the last to the front of Mr. Partridge's	
shop	890
From Arsenal Compound to between Hospital Na-	
tive Infantry and the Executive Engineer's Office	
Compound	1,300

		Feet.
	From last to the Sappers and Miners' Lines From Arsenal Compound to the corner of the Gym	2,420
	Khana	1,860
	cellency the Commander in Chief's Compound From corner of Gym Khana to open space near	2,100
	Compound No. 19	1,000
	From Compound No. 19 to the rear of the Revenue Commissioner's Office	1,100
	Total length in feet	17,171
	Or in miles, yards, and feet 8 m. 1,643 yds. 2 f	t.
	(Signed) PHILLE L. H. Captain Engineers, on Speci	
P	Poona, October, 1857.	•

POONA DIVISION.—PUBLIC WORKS.

CIVIL.

Estimate framed by Captain P. L. Hart, of the Engineers, on special duty, of the probable expense of supplying the Poona Cantonment with water from the Ambeygaum Valley. Estimate framed agreeably to instructions contained in the Government Resolution, under Mr. Secretary Hart's Memorandum No. 160, of the 17th January, 1857.

1. Embankment 1,52,064 2. Waste weir 10,517 3. Artificial cut to carry off first floods 24,651 4. Inlet tower and filter 12,172 5. Gangway 9,082 6. Masonry aquedue and tunnel 1,14,969 7. Iron conduit pipe and tunnel 1,85,434 8. Distribution reservoir, for two days' supply 36,834 9. Distribution reservoir, for one day's supply 22,599		Rupces.	
3. Artificial cut to carry off first floods 24,651 4. Inlet tower and filter 12,172 5. Gangway 9,082 6. Masonry aqueduc and tunnel 1,14,969 7. Iron conduit pipe and tunnel 1,85,434 8. Distribution reservoir, for two days' supply 36,834	1. Embankment	1,52,064	
4. Inlet tower and filter 12,172 5. Gangway 9,082 6. Masonry aqueduc and tunnel 1,14,969 7. Iron conduit pipe and tunnel 1,85,434 8. Distribution reservoir, for two days' supply 36,834	2. Waste weir	10,517	,
5. Gangway 9,082 6. Masonry aqueduc and tunnel 1,14,969 7. Iron conduit pipe and tunnel 1,85,434 8. Distribution reservoir, for two days' supply 36,834	3. Artificial cut to carry off first floods	24,651	
 Masonry aqueduc and tunnel	4. Inlet tower and filter	12,172	
7. Iron conduit pipe and tunnel 1,85,434 8. Distribution reservoir, for two days' supply 36,834	5. Gangway	9,082	
8. Distribution reservoir, for two days' supply 36,834	6. Masonry aqueduc and tunnel	1,14,969	
	7. Iron conduit pipe and tunnel	1,85,434	
9. Distribution reservoir, for one day's supply 22,599	8. Distribution reservoir, for two days' supply	$36,\!834$	
	9. Distribution reservoir, for one day's supply	$22,\!599$	
10. Camp distribution 1,03,409	10. Camp distribution	1,03,409	

GENERAL DESCRIPTION.

The general description of the scheme now submitted is as follows:—
The construction of an earthen dam across the gorge of the valley near the village of Upper Ambeygaum, 1,270½ feet in length, its greatest height above the bed of the stream being 59.86 feet. From the reservoir so formed the water will be led, by either an iron conduit pipe, or masonry aqueduct, whichever may be considered the best and most economical, to the most favourable point for crossing the small plateau of the spur dividing the Kartriz and Ambeygaum valleys. Through this portion a tunnel will be driven 2,781½ feet long, opening at the head of the Duncowree Valley. The course then,

for either pipe or masonry aqueduct, will be down this little valley for some short distance. If by an iron conduit pipe, it will run almost straight to a distributing reservoir, to contain either one or two days' supply, near the solitary cells in the Camp of Poona; if by a masonry aqueduct, before reaching the mouth of the Duncowree Valley, it will branch off, running slightly up the Kartriz Valley, crossing the aqueduct, round under the village of Beebee Warree, and thence following the sinuosities of the hills to the distributing reservoir above mentioned, from which water can be delivered by gravitation to every part of the Cantonment, wherever required. by a masonry aqueduct, the length will be 7 m. 6 fur. 651 yds.; if by an iron pipe, it will be 5 m. I fur. 94% yds., the total fall from the bottom of the main reservoir to the Camp distribution reservoir being 80:25 feet. The distributing reservoir, as above observed, will either contain one or two days' supply, and from it the water will be conveyed, by iron pipes of four and five inches diameter, to cisterns corresponding in number and capacity with the wants of the locality, with a stand-pipe at each eistern, the pipes always remaining charged. Plans Nos. 1 and 8 show these works.

EMBANKMENT.

The site occupied by the embankment, the details of which are shown in the Plan No. 2, to be cleared of all loose soil or soft rock, so that it may rest on the solid soil or clay, to the average depth of one foot over the whole surface so occupied; and on approaching the hill sides at each end, the ground to be regularly stepped or leached, in order that these portions may rest on a fair and horizontal base. An excavation for the trench intended to receive the puddle to be taken down to such a depth as will enable the puddle to rest either on solid rock, or on such a permanently secure base as will effectually prevent all leakage of water. The trench will also be stepped into the hill sides, in order that the puddle may have a fair bearing The dotted line coloured yellow in Plan No. 2 shows the probable depth that the puddle trench will require to be excavated, and was ascertained from pits sunk at certain distances apart throughout the whole length of the embankment. The trench to be filled in with clay, well watered, in layers of six or eight inches in depth, and each layer to be well worked up with the feet. Particular

care must be taken to ensure its thorough consistency throughout, by always keeping the surface left well watered, so as to obtain a thorough junction with the next layer it may receive; this must extend throughout the whole puddle wall. It is to be eight feet wide at top, carried up to the summit of the embankment, and increasing in width at the rate of four feet for each ten feet of depth. All stones of more than half a pound weight, or other extraneous matter than good plastic clay, to be rejected, and all lumps and clods to be broken up and pounded, before being worked into the trench. There must be a uniform consistency throughout in this portion of the work, and no joints, scams, or cracks of any sort or description whatever allowed. The puddle wall and embankment to be carried on simultaneously, and no one portion of the puddle allowed to rise above the adjoining portion of the embankment.

The embankment to be twenty feet wide at top, with a slope on the inside of 3 to 1, and on the outside of 2 to 1, formed of the materials to be found close at hand; the best or most clayey portion to be put in the middle next to the puddle wall on the upstream side, and the least clayey on the outside half of the embankment. exceeding three inches in diameter to be rejected. The embankment to be formed in parallel layers of about nine inches in thickness, inclining from both faces towards the puddle wall at a slope of 1 in 10. Each layer to be well watered, and pounded with wooden rammers, so that it may be thoroughly consolidated, the layers being kept of a uniform thickness throughout. The internal slope, the top, and for a length of ten feet of the external slope of the embankment, to be covered with dry stone pitching set carefully by hand, each stone on its end, two feet in depth, and resting on a bed of quarry shivers and broken stone, nine inches deep. The stones of which the pitching is formed to be of a general uniform thickness throughout, not tailing to a point, but with a fair, but rough, bed throughout the whole depth of two feet, and resting on an end similar to the face; the interstices to be carefully filled in with stone chippings. The face of this rough stone pitching to have an even and true surface throughout, and not to have that wavy appearance which bad work would denote. The surface to be as even as a well-built rubble masonry wall. A small rubble masonry wall to run along the foot of the internal slope (see Plan No. 2), the too being on a level with

the ground, for the rough stone pitching to abut against. This wall will tend to prevent any uneven settlement of the pitching, and preserve for it a true and even face.

THE WASTE WEIR.

A waste weir to be carried round the eastern end of the embankment, as shown in Plans Nos. 3 and 6, by an excavation in the solid ground 841 feet in length, and 100 feet in breadth, communicating with the "artificial cut to carry off the first floods" at its lower end The side slopes of this channel have been calculated at 1 to 1, but, as the excavation throughout will most probably be in moorum, so large a slope will not be necessary; a deduction has consequently been made in the estimate of 25 per cent. from the solid contents. At the lower end of the waste weir, a masonry apron, with dwarf walls on each side, is to be built, to prevent the water working a channel for itself between the masonry of the apron and the adjoining soil. The foundation of the apron to be of uncoursed rubble masonry, with a rough slab pavement set on end in lime above it, two feet thick. The cross walls, at the upper and lower end of the apron, to be sunk below the foundation, to prevent the water from undermining it in any way.

ARTIFICIAL CUT TO CARRY OFF THE FIRST FLOODS.

The artificial cut is shown in Plans Nos. 3 and 6 in detail. It is to be 7,900 feet in length, with a breadth at base of 40 feet, side slopes calculated at ½ to 1, but the same deduction to be made, on the account above stated. On leaving the bed of the Ambeygaum Nullah above the dam it runs for a length of 2,739 feet with a fall of 4.92 inches per 100 feet; for 1,615 feet with a fall of 1.85 inch to 100 feet; for 1,842 feet with a fall of 0.81 inch to 100 feet; for 715 feet with a fall of 6.29 inches to 100 feet, joining the stream again behind the embankment with a fall of 6.97 inches to 100 feet, in a length of 987 feet. This cut, crossing the three tributaries of the main stream, will require three small masonry dams to be built at the points of intersection, in addition to one across the main Nullah, at the point the artificial cut leaves it. The foundations of these dams to be carried down to the solid rock, to be built of uncoursed rubble masonry; the breadth of the dam at bottom to be of a thick-

ness equal to the height, and the top to be half the thickness at bottom, all to be built of uncoursed rubble masonry. Openings to be left in each, to enable the clear water to feed the reservoir after the first heavy monsoon muddy floods have passed off. For a further description, see paras. 46 to 49 of the Report accompanying.

INLET TOWER AND FILTER.

The details of this work are shown in Plan No. 5. The foundation of the inlet tower to be carried down 13 feet below the bed of the Nullah, or for such a further depth as may be found necessary, until a good foundation or solid rock be reached. On the foundation so excavated, a layer of bétou to be laid over the whole extent (33 feet square), two feet deep, and on this a half-spherical invert to be turned of roughly-dressed stones, two feet deep, set in lime in concentric circles, having an inner diameter of 13 feet. • The work on the outside of the invert to be brought up in eight circular offsets of one foot in depth, and decreasing half a foot in breadth, of coursed rubble masonry, leaving the foundations at top 51 feet thick, on which the walls of the tower Openings to be left in the foundations and walls, as described previously, in para. 53 of the Report accompanying. On this halfspherical invert, the walls of the tower are to be carried up to a height of 60 feet, 5 feet thick at bottom, and 21 feet thick at top, of coursed rubble masonry faced with cut-stone on both sides, particular care to be taken in the beds and joints, which are to extend at least one foot into the walls, and to be very close and fine, so as to secure a watertight junction. At a height of about 15 feet above the foundation of the tower, there is to be a domed floor, set in concentric rings, of cut-stone masonry; the space included between the invert and this domed floor is the filter, the bottom part to be filled with amygdaloid,* and the upper portion with layers of coarse and fine sand. To communicate with the plugs in the iron tube, a circular staircase of roughly dressed stone, each stone to be well tailed into the walls of the tower, runs from the top of the tower to the doined floor. iron tube, 3 feet in diameter, passes down the centre of the tower,

^{*} A Scotch engineer, Mr. Thom, of much experience in works of this nature, found this species of friable trap rock, which abounds in the Deccan, the best substitute for animal charcoal, which is one of the most perfect filtering media.

and through the domed floor, provided at every two feet with spouts six inches in diameter, into which the wooden plugs are inserted for the supply of the tube, and winding spirally round it, to allow of their being easily closed or opened from the steps. A cut-stone cornice runs round the top of the tower, above which the walls are carried up 10 feet high, of coursed rubble masonry, with circular openings on three sides, covered with a teak-wood roof and double tiles. For any further particulars regarding this work, reference is requested to para. 53 of the Report accompanying, in which it is fully described. Ties of iron passing through the walls of the tower may, probably, be required to keep the tube firmly in its position; but, as the water in the upper part of the tower will always retain the same level as that outside, its pressure alone will probably be sufficient for this purpose. This subject is mentioned to show that it has not been lost sight of, though it is not thought necessary to provide for it in the estimate.

GANGWAY.

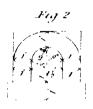
The foundation of the gangway pier, to be taken down to a depth of 10 feet, or such further depth as may be found necessary until a solid foundation is reached (Plans Nos. 2 and 4 show the details of this work), and to be filled in with rubble masonry. The superstructure to be carried up in three offsets of 195, 20, and 17 feet respectively, of coursed rubble masonry, the thicknesses of the pier being respectively 6, 5, and 4 feet, in each division of its height. The pier, or abutment in the embankment, which will not be built until the lower portion has thoroughly consolidated, to be 12 feet in height, with a thickness of 4 feet, of coursed rubble masonry. The trusses of the gangway to be constructed of well-seasoned teak, procured for the most part from Bombay, free from cracks and knots, the Deccan timber not affording the length required. The castings to be clean and sound, and the wrought-iron work executed in a true and workmanlike manner, particularly the cutting of the threads of the screwbolts and nuts, which are to be sharp and clean, with an even and true bearing for the nuts. All the joints of the wood-work to be close and truly fitted. The tongues between the timbers of the upper and lower chords to be of well-seasoned babool-wood, closely jointed. For further particulars of this work, see the description of it at para 55 of the Report accompanying.

MASONRY AQUEDUCT AND TUNNEL.

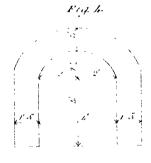
The total length of the masonry aqueduct, from the inlet tower to the distribution reservoir in the Camp, is 41,116 feet, or 7 miles 6 furlongs $65\frac{1}{3}$ yards, divided into the following portions (Plans Nos. 7 and 8 contain the details of this work):—

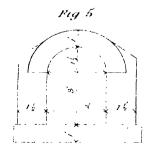
	Length.	Inclination.
From the inlet tower to the mouth of the tunnel on the Ambeygaum side, the general section of the aqueduct is that shown at Fig. 1, except for a short distance where the aqueduct passes under the embankment (335 feet in length). Fig. 2 shows this section: it is precisely similar to that above, with the exception of the top being arched over, to prevent the weight of the embankment from crushing it in, which might probably occur with a section like Fig. 1 The section of the tunnel is similar to Fig. 3, of sufficient size to allow of two men driving it; where the tunnel joins the entrance shafts Nos. 5 and 8 (of Plans 7 and 8) a small extent of the length at the junction will probably require revetting, as shown in Fig. 4 Fig. 5 is the section of the aqueduct between chambered shafts Nos. 1 and 5 of Plan No. 8, viz. that portion of it which descends the Duncowree Valley On account of the considerable fall, and the probability that a large quantity of water can be stored in this part of the aqueduct during the hot weather, it has been made of a larger and stronger section. It is highly probable that there will be no necessity, in many	Feet. $4,729\frac{1}{2}$ $2,781\frac{1}{2}$ 2.570	Level. Feet. Feet. 0.84 to 100











	Length.	Inclination.
parts, for either a masonry sill or side walls, as the excavation will be in rock; it has, however, been considered best to provide for it in the estimate. The same remark will apply to the aqueduct, having sections similar to Figs. 1 and 2. From No. 1 chambered shaft to the distribution reservoir in the Camp, the section is similar to Fig. 1, which is the general section of the aqueduct throughout, it only having been departed from in the places where it has been considered as	Feet. 1,397	Feet. Feet. 1.81 to 100
actually necessary	29,638	Inch. Feet.

- (Fig. 1.) This part of the aqueduct to be constructed of coursed rubble masonry, with slab-stones set in lime for the sole and top of the aqueduct.
- (Fig. 2.) That portion passing under the embankment will be built in a similar manner to the above, with the exception of an arched head instead of a flat one.
- (Fig. 3.) The section of the tunnel will be as shown in this figure, excepting those parts adjoining the entrance shafts, marked Nos. 5 and 8 of Plan No. 8, which will be revetted with coursed rubble masonry, for a distance at the junctions of 20 feet (see Fig. 4). The two shafts at the mouths of the tunnel, and the air-shafts, will be built of coursed rubble masonry.
- (Fig. 5.) This section of the aqueduct will be constructed of coursed rubble masonry, the chambered shafts (Nos. 1, 2, 3, and 4) being of the same construction, but faced with cut-stone on the inside.

The trench for the aqueduct to be half filled in with the soil excavated from it, when that part of the work is finished.

IRON CONDUIT PIPE AND TUNNEL.

The iron conduit pipe leaves the inlet tower for a length of $4,729\frac{1}{2}$ feet, at a slope of a quarter of an inch to 100 feet to the mouth of the tunnel, through which it is laid level for a length of $2,781\frac{1}{2}$ feet. It then descends at a slope of 0.84 foot to 100 feet, for a distance of

2,570 feet, to the chambered shaft (No. 3) covering the spring, shown in Plans Nos. 7 and 8. The water entering one compartment of the shaft is discharged into the other, from the bottom of which the pipe proceeds, with various rises and falls, to the distribution reservoir in the Camp, the head of water to overcome which is 31 feet. For a further description of this portion of the work, see paras. 56 and 57 of the Report accompanying; Plan No. 7 showing the details, and Plan No. 8 a longitudinal section of the conduit pipe, from the bed of the Nullah at Ambeygaum to the distribution reservoir.

DISTRIBUTION RESERVOIRS (TWO AND ONE DAY'S SUPPLY).

Details of these works will be seen in Plans Nos. 6 and 7. foundations of the retaining walls to be of uncoursed rubble masonry, faced with cut-stone on the inside. The foundation of the surrounding wall of the reservoir to be of the same construction as the above; the superstructure to be built of coursed rubble masonry with a cutstone coping, and to be furnished with two strong teak-wood plank battened doors. The space between the foundations of these two walls, particularly to the sloping portions on the north side, and the lower parts of the east and west ends, to be filled in with earth in regular layers, well rammed, to prevent leakage. The bed of the reservoir throughout to be paved with cut-stone. In the event of this head of water being made use of for flushing sewers or drains (to be hereafter built throughout the Cantonment), two sluices will be required; they have not been included in the estimate, as they do not properly belong to the water supply of the Camp. For further particulars regarding these works, see paras. 62 and 63 of the Report accompanying, and letters E and F of the Appendix to it.

CAMP DISTRIBUTION.

The retaining walls of the cisterns to be built of coursed rubble masonry, with a cut-stone facing on the inside; a parapet wall of cut-stone to surround the cistern at the top. The bottom of the cistern, and the part surrounding the parapet wall at top, to be paved with cut-stone. The iron pipes conveying the water from the distributing reservoir to be of the kind usually denominated socket-pipes; to be perfectly sound, and free from honeycombs or other defects in the casting, with a stand-pipe and stop-cock provided for each cistern. The details of the Camp distribution are shown in Plan No. 9.

No. 1.—THE EMBANKMENT.

MEASUREMENTS.

Stepping the Embankment into the Sides of the Hill.

	PLAN No. 2.	No.	Length.	Breadth.	Depth.	Solid Feet.
	(RIGHT SIDE.)					
lst I	Portion	1	9.5	23:41+20	1.25	128.97
2nd	do	1	10	20:85 +28:24	1.52	157.15
3rd	do	1	10	30-26+36-85 2	1:25	178.46
4th	do	1	10	33 67 + 30.26	1.25	199.78
5th	do	1	10	37:05 + 33:67	1.25	221.09
	do	1	10	40.40 + 37.08	1.25	242.40
7th	do	1	10	43.9 + 40.49	1.25	263.71
8th	·do	1	10	59·91 + 13·9	1.67	429.23
9th	do	1	10	73:92 + 58:91	1.07	554.56
10th	do	1	10	88 95 + 73 92	1.67	679.98
11th	do	1	10	98-96+84-95	$\frac{2}{2}$	939.55
12th	do	1	10	108 97+06:00	3-2	1,559.47
13th	do	1	10	118-98+108-97	$\frac{3}{2}$	1,709.62
14th	do	1	10	185-15+118-98	3	1,905.97
15th	do	1	10	185·15+156·45	5 2	3,645.00
16th	do	1	10	$\frac{156\cdot 15+168\cdot 52}{2}$	$\frac{3}{2}$	2,437.27
17th	do	1	10	168·52+171·36	$\frac{1}{2}$	849.70
18th	do.	1	10	171:36+181:78	$\frac{2}{2}$	1,765.70
19th	do	1	10	181·78+199·75 2	4 2	3,815.30
	(LEFT SIDE.)					
1st P	ortion	1	9.25	20.0+33.14	1.3	185.37
2nd	do	1	10	33·4·1+46·88 2	2.5	502.00
3rd	do	1	10	40:88+00:32	2.5	670.00

Stepping the Embankment into the Sides of the Hill—(continued).

	No.	Length.	Breadth.		Solid Feet.
				l	•
4th Portion	1	10	60·32+73·76 2	3.5	1,173-20
5th do	1	10	73.70+87.20	3.5	1,408.40
6th do	1	10	87·20+100·65	3.5	1,643.68
7th do	1	10	100-05+122-52	4 2	2,231 70
8th do	1	10	122-52+144:30	4.5	3,002.73
9th do	1	10	144·89+166 26 2	4.5	3,494 81
10th do	1	01	108:20+188:13	$\frac{45}{2}$	3,986.88
11th do	1	10	188-13+210-00	$\frac{3.5}{2}$ -	3,483 63
12th do	1	10	210-00+210-85	1·5 2	1,611-93
13th do	1	10	219.85 + 229.70	0.2	561-93
14th do	1	10	229.70 + 239.55	2 2	2,346 25
15th do	1	10	$\frac{239\ 55 + 249 \cdot 40}{2}$	2.5	3,055 93
16th do	1	10	240-40+250-25	2 -2	$2,543 \cdot 25$
Total solid feet stepping to	53,584 ·60				

Stripping Loose Earth from the Bed of the Embankment.

PLAN No. 2. (from the left.)	No.	Length.	Breadth.	Depth.	Solid Feet.
1st Portion	1	58.5	259 25 + 278·90 2	1	15,740.88
2nd do	1	116	$\frac{278\cdot90+335\cdot80}{2}$	• 1	35,652.60
3rd do	1	6	355-80+311-25	1	1,941·15
4th do	1	18.5	$\frac{311\cdot25+319\cdot30}{2}$	1	5,832.58
5th do	1	32	$\frac{319:30+314:95}{2}$	1	10,148.00
6th do	1	14	$\frac{314.05+295.05}{2}$	1	4,276:30
7th do	1	40	295.95+292.45	1	11,768.00
8th do	1	68.5	202-45+270-25	1	19,477.97

Stripping Loose Earth from the Bed of the Embankment—(continued).

	No.	Length.	Breadth.	Depth.	Solid Feet.
9th Portion	1	35.5	270·25+278·05 2	1	9,838.82
10th do	1	47.5	278-05 + 280-65 2	1	13,411.62
11th do	1	53.5	286 65 + 205:15	1	15,563:15
12th do	1	46.5	205:15+201:75	1	13,695·17
13th do	1	11	204.75+306.60	1	3,307.42
14th do	1	24	$\frac{306:80+289:55}{2}$	1	7,153.80
15th do	1	88.5	280:55+270:85	1	25,195·95 *
16th do	1	9	279:85 + 286:45 2	1	$2,\!458\cdot\!35$
17th do	1	72.0	266-17+250-85	1	18,946.80
18th .do	J	9.5	259·85 + 248·10	1	$2,\!390.43$
19th do	1	114	243·40+219·85 2	1	$26,\!405.25$
20th do	1	57	$\frac{219.85 + 199.75}{2}$	1	11,958.60
Total solid feet stripping	g ear	th from	the bed	of the	
embankment					255.162:84

Exeavating Foundation for the Dwarf Wall for the Pitching to abut against.

	No.	Length.	Breadth.	Depth.	Solid Feet.
Dwarf wall at the foot of the embankment, upstream side	1	1,377:38	3.66	4	20,164.84
Total solid feet of excavat					

Excavating Trench to receive the Puddle.

(F	PLAN No. 2.	No.	Length.	Breadth.	Depth.	Solid Feet.
•	Portion	1	11	8+9.87	5·0 1	463.92
2nd	do	1	24	8-97+11-51	5.0	1,224.60
3rd	do	1.	25.5	11:54+14:45	50	1,656.86
4th	do	1	12.5	$\frac{14.45 + 16.68}{2}$	5:0	972:81
5th	do	1	22	16.6° + 20.74	5:0	2,058·10
6th	, do	1	18.5	20·74+23 <u>·67</u>	5:5+5 2	$2,\!156.66$
7th	do	1	17:25	23:67 + 24:16	5+7	$2,578\cdot34$
8th	do	1	28.5	21.16 + 27 14	7:0	5.1.17.17
9th	do	1	58.5	27·14 + 29·71	7 ± 9·5	13,477:30
10th	do	1	116	28:71 + 28:86	9.5 + 12.5	36,729.66
11th	do	1	6	28:86+31:30	125+105	$2,075\cdot52$
12th	do	1	18.5	31 30 + 31.04	105+05	5,849.70
13th	do	1	32	31 94+31.59	9:5+11	10,418.92
14th	do]	14	31·59 + 30·07	$\frac{11+14.5}{2}$	5,503:15
15th	do		40	30.07 + 29.79	$\frac{14.5 + 15}{2}$	17,658.70
16th	do	1	68.5	20 79 + 28·50 2	$\frac{15+19}{2}$	33,939·35
17th	do	1	35.5	28 50 +28:64	19+18·5	19,016.90
18th	do	1	47.5	28·64 + 29·33 2	18:5+17:5	24,782.17
19tli	do	1	53.5	29:33 + 30 01	17:5+15	25,794.35 *
20th	do	1	46.5	30-01 + 29-98	15+15	20,921.51
21st	do	1	11	29.09 + 30.92	15+12.5	4,605.56
22nd	do	1	24	30·92+29·56	12.5+16	10,342.08
23rd	do	1	88.5	$\frac{29.50 + 28.78}{2}$	16+14.5	$39,\!368\cdot\!56$
24th	do	1	9	28 78 + 27.71	14.5+16	$3,\!876.62$
25th	do	1	72	27-71+27-18	$\frac{16+14.5}{2}$	30,134.61
26th	do	1	9.5	27·18+25·87	14.5+17	3,968.80

Excavating Trench to receive the Puddle—(continued).

	Exco	ivating Trencl	n 10 1 No.	receive th Length.	e Fuaate Breadth.	—(<i>cont</i> Depth.	Solid Fect.
27th I	Portio	n	1	114	25:87 + 23:08	17 + 11·5 2	40,490.66
28th 29th	do. do.		1	57 15	5 55-38+50-55 53-04+55-38	$\frac{11}{2}$ $\frac{10+10}{2}$ $\frac{10+13}{2}$	14,203.54 $3,514.50$
3 0th	do.		l	19	20·22 + 19·76	12+9	3,798·10
31st 32nd	do. do.		1 1	20·5 34·5	10.70 + 16.36 2 16.36 + 13:51	$\frac{8}{1}$ $\frac{6+5\cdot5}{2}$	2,961·84 3,477·98
33rd	do.		. 1	30	13:51+11:11	5:5	2,031·15 4,269·89
34th Total s	- do. solid . 1	eet excavating	l trei	71·5 ch_to_red		55+7 middle	4,2000
•		pebbles, and w					3,99,418.50
	Bu	ilding $m{D}war\!f$	Wal	l for Pit	ching to a	but aga	inst.
			No.	Length.	Breadth.	Depth.	Solid Feet.
Footin	g	•••••	1	1,377:3	8 3.66	1	5,041:21
Wall,	1st pe	ortion	1	1,377:3	8 2.66	1	3,663.83

	110.	mengen.	meauth.	Depen.	Soud I ((),
Footing	1	1,377:38	3.66	1	5,041:21
Wall, 1st portion	1	1,377:38	2.66	1	3,663.83
,, 2nd do	1	1,377:38	$\frac{2.60+2}{2}$	2	$6,\!418.59$
Total solid feet of building	g dwa	arf wall fo	or the p	itching	
to abut against, of unco	ursec	l rubble m	asonry	• • • • •	15,123.63

Filling in Puddle Trench with Clay.

No. Length. Breadth. Depth. Solid Feet.

Same as the excavation above.....

3,99,418.50

Total solid feet of filling in puddle trench with clay 3,99,418.50

Constructing Puddle Wall of Embankment above the Trench filled in with Clay.

			No.	Length.	Breadth.	Depth or Height.	Solid Feet.
1st P	ortion	ı	1	11	***7+8 +	2.18	98:58
2nd	do.		1	24	11 54+8 , 8 97+9	8.87+ 2.18	1,206.99
3rd	do.	,	1	25.5	14 15 + 8 + 11:54 + 5	16:13+ 8 87	3,346.07
4th	do.		1	12.5	16 68+9 + 14.45+8		2,787:29
5th	do.		1	99	20:71+8 + 16:68+9	31-85 + 21.72	7,869:70
6th	do,		1	18.5	$\frac{23.67+8}{2} + \frac{20.71+4}{2}$	39-18+31-85	* 9,932.75
7th	do.		1	17:25	$\frac{24\cdot16+8}{2} + \frac{23.67+9}{2}$	40 4 +39.18	10,952.86
Sth	do.		1	28.5	27:14+4 + 21:16+4	47:85 + 10:4	21,158-48
:)th	do.		1	58:5	24·7] + 7 + 27·11 + 8	51:78 + 47:85	52,345.91
10th	do.		1	116	$\frac{28\cdot80+8}{2} + \frac{28\cdot71+}{2}$	57:16+51 78	116,213:37
11th	do.		1	6	31-30+x + 2x x6+8	58:25+57:10	6.592.21
12th	do.		1		$\left \frac{31.51 + 8}{31.30 + 8} + \frac{3}{31.30 + 8} \right $	_	21,642.77
13th	do.		1	32	$\frac{31.59+8}{2} + \frac{31.91+8}{2}$		
14th	do.		i	14	30·07+8 + 31·50+8		15,517.63
15th	do.		1	40	$\frac{29.70+8}{20} + \frac{30.07+8}{2}$	54· 49 +55·19	41,601.62
16th	do.		1		$\frac{26.5+8}{2} + \frac{29r79+8}{2}$		67,262.08
17th	do.		1		$\frac{28.01+4}{28.01+4} + \frac{28.5+8}{2}$		33,384·11
			. 1		$\frac{2}{30\cdot 33+8} + \frac{5}{30\cdot 61+4}$		46,106.88
18th	do		1	47.5	2 + - 2	53-53+51	46,106.88

Constructing Puddle Wall, &c.—(continued.)

	No.	Length.	Breadth.	Depth or Height.	Solid Feet.
19th Portion	1	1	$\frac{30\ 01+8}{2} + \frac{29\cdot33+8}{2}$		54,595.69
20th do	1	46.5	30 05 + 4 + 30 01 + 8	1:954-55:03	48,577.27
21st do	1	11	ν±αθ-υΣ , ν±ζυ (υ.	7:52+54:95 2	11,871·14-
22nd do	1	24		3.91 + 57 3?	25,520.61
23rd do]	88	28.7848 29.5649		87,074.25
24th do	1	9			8,257.87
25th do]		35.18 + 8 + 25.21 + y	17·97 + 19·29	62,052.85
26th do	1	9.5	27.87 + 9 27 14 F4	14 68+ 17 97	7,597.01
27th do	1	114	23.95 + 5 25.87 + 5	39-97 + 14-68	79,432.38
28th do	1	57	25.38 + 8 + 5.3.08 + 8	35:95+39.97	33,732.39
29th do	I	15	20:29 +8 + 22:384 5	30•56 3 5 95	7,307.78
30th do	1	19	$\frac{19.76+8}{2}$ + $\frac{20.22+}{2}$	29-49-4-30-56	7,974.49
31st do	l	20:5	16:36+8 1 19:76+8	20-00 F 20-42	6,720.61
32nd do	1	34.5		13:79 + 20:90	6,865·1 7
3 3rd do	1	30	$\frac{11\cdot11+8}{2} + \frac{13\cdot51+8}{2}$	× 78+13·79	3,437.97
34th do.	1	70.5	$\frac{90.84.8}{2} + \frac{11.11+8}{2}$	0.00+8.78	2,716.59
Total solid feet construent	uctin	g puddl	le wall of c	embank-	9,49,551.93

Covering Internal Slope and Top of Embankment with Rough Stone Pitching.

			No.	Length.	Breadth or Slope.	Square Feet.
Intern	NAL SL	OPE OF EMBANKMENT.		I i		1
					Hypotenuse.	
1st P	ortion	1	1	11	6.80+0.00	37.89
2nd	do.		1	24	28.04+6.89	419.16
3rd	do.	`	1	25.5	51.00+28.04	1,007.76
4th	do.		1	12.5	2 *	753.00
5th	do.		1	22	100-09+69-48	1,871.87
$6 ext{th}$	do.		1	18.5	$\frac{123.88 + 100.69}{2}$	2,077.27
7 h	do.		1	17:25	127.73 + 123.88	2,170.13
8th	do.		1	28.5	161-28+127-78	3,975:89
9th	do.		1	58.5	$163 \cdot 72 + 151 \cdot 28$	9,213.75
10th	do.		1	116	$\frac{180 \cdot 72 + 163 \cdot 72}{2}$	19,977.52
11th	do.		1	6	181-16+180-72	1,094.64
12th	do.		1	18.5	180-20 + 184-16	3,454.41
13th	do.		1	32	186-54 + 180-20	6,013.28
14th	do.		1	14	174·49+186·54	2,527.20
15th	do.		1	40	172-29+174-49	6,935.40
$16 ext{th}$	do.		1	68.5	$\frac{162 \cdot 04 + 172 \cdot 28}{2}$	11,450.46
17th	do.		1	35.5	$\frac{163 \cdot 18 + 162 \cdot 04}{2}$	5,772.65
18th	do.		1	47.5	$\frac{168.02 + 103.18}{2}$	7,880.25
19th	do.		1	53.5	$\frac{174 \cdot 00 + 168 \cdot 62}{2}$	9,165.08
2 0th	do.		1	46.5	173·73+174·00	8,084.72
21st	do.	,	T	11	155-23+173-73	1,974.28
22nd	do.		1	24	$\frac{170\cdot 44 + 185\cdot 23}{2}$	4,268.04
. 2 3rd	do.		1	88.5	164 32 + 170.44	14,813 ⁻ 13
24th	do.		. 1	9	$\frac{155 \cdot 84 + 164 \cdot 32}{2}$	1,440.72
25th	ďo.	•••••	1	72	151·87 + 155·84 2	11,070.36

Covering Internal Slope, &c.—(continued.)

Į	No.	Length.	Breadth or Slope.	Square Feet.
INTERNAL SLOPE OF EMBANKMENT.			Hypotenuse.	
26th Portion	1	9.5	141 20 + 151·67 2	1,391.41
27th do	1	114	126 38 + 141 20 1 2	15,255.48
28th do	1	57	113.67 + 120.38	6,841.42
29th do	1	15	$\frac{96.62 + 113.67}{2}$	1,577-17
30th do	1	19	03:02 + 90 02	1,801.58
31st do	1	20.5	60.08+03.02	1,630.77
32nd do	1	34.5	43.60+ 66.08	1,891.98
33rd tlo	1	30	27.76+43.60	1,070.40
3 4th do	1	70.5	0.00+27.70	978.54
Top of embankment	1	1270-25	20 1	25,40500
Part of external slope	1	1230-16	1 10 1	12,301.60
, · · · · · · · · · · · · · · · · · · ·		r	•	

Total square feet covering internal slope and top of embankment with rough stone pitching, on a bottom of quarry shivers and loose small stones nine inches deep. 202,594.22

Forming Embankment.

	No.	Length.	Area of Prismoid.	Solid Feet.
1st Portion	. 1	70.5	152.04	10,718.82
2nd do	. 1	30	549:30	16,479.00
3rd do	. 1	34.5	1,109.554	38,279.61
4th do	. 1	20.5	2,100.887	43,068.18

Forming Embankment—(continued).

			No.	Length.	Area of Prismoid.	Solid Feet.
5th	Portion		1	19	2,848.571	54,122.84
6th	do.		1	15	$3,\!435.89$	51,538:35
7th	do.		1	57	4,364.97	2,48,803:29
8th	do.		1	114	5,329.635	6,07,578.90
9th	do.		1	9.5	6,293.769	59,790.80
10th	do.		1	72	6,885:155	4,95,731.16
11th	do.		1	9	7,422.588	. 66,803.29
12th	do.		1	88.5	8,066:193	7,13,858.08
13th	do.		1	24	8,847:292	2,12,335.00
14th	do.		1	11	9,001:715	99,018.86
15tb	do.		1	46.5	8,659.551	4,02,669.12
16th	do.		1	53.5	8,422.883	4,50,624.24
17th	do,		1	47.5	7,932.768	3,76,806.48
18th	do.		1	$35 \cdot 5$	7 ,641·239	2,71,263.98
19th	do.		1	68.5	8,047.679	5,51,266.01
20th	do.		1	40	8,615.466	3,44,618.64
21st	do.		1	14	9,292.978	1,30,101.69
22 nd	l do.		1	32	10,016.983	3,20,543.45
2 3rd	do.		1	18.5	$9,900 \cdot 372$	1,83,156.88
24th	do.		1	6	9,479.014	56,874.08
25th	do.		1	116	8,512.882	9,87,494.31
26th	do.	,	1	58.5	7,203.352	4,21,396.09
$27 ext{th}$	do.		1	28.5	5,761.602	1,64,205.65
28th	do.		1	17.25	4,754.22	82,010.29
2 9th	do.		1	18.5	3,874.781	71,683.44
30tl	do.		1	22	2,350.668	51,714.69
31st	do.	• • • • • • • • • • • • •	1	12.5	1,280.399	16,004.98

Forming Embanhment—(continued).

	No. Length.	Area of Prismoid.	Solid Fo	eet.
32nd Portion	1 25.5	651.605	16,61	5.92
33rd do	1 24	$196 \cdot 138$	4,70	7:31
34th do	1 11	25.76	28	3.36
Deduct puddle wall of embank	ment as abov		949,55	1.93
Total solid feet for	ming emban	kment	6,672,61	4.86
Α	BSTRACT.			
Quantities.			Rs. a.	p.
53,584 Solid feet stepping the hill sides in per 100 solid fee 2,55,162 Solid feet strippin bed of the emba	moorum, at R et ng the earth	s. 0-10-6 from the	351 10	3
per 100 solid fee 20,164 Solid feet of exca at the foot of	et vation for dy	varf wall	637 14	5

stream side, in earth and gravel, at

ceive the puddle in earth, gravel, and water, at Rs. 2-8-0 per 100 solid feet.

pitching to abut against, of uncoursed rubble masonry, at Rs. 8 per 100

88

1,209 13

9,985

3 5

2

5

Rs. 0-7-0 per 100 solid feet

3,99,418 Solid feet of excavation for trench to re-

15,123 Solid feet of building dwarf wall for the

solid feet

ABSTRACT—(continued).

Quantities.	$\mathbf{R}\mathbf{s}.$	a.	p.
3,99,418 Solid feet of filling in puddle trench with clay, at Rs. 1-2-0 per 100 solid feet	4,493	7	2
9,49,551 Solid feet constructing puddle wall of embankment, at Rs. 1-2-0 per 100			
2,02,594 Square feet of covering internal slope of	0,682	7	2
embankment, &c. with rough stone pitching set on end, on a bottom of			
quarry shivers and small stones nine inches thick, at Rs. 0-4-0 per square foot	0,648.	Q	0
66,72,614 Solid feet of forming embankment, at	•		
Rs. 1 per 100 solid feet 66	3,726.	2	2
Total 1,44			
Contingencies, at 5 per cent	7,241	2	10
Total amount for the embankment Rs. 1,55	2,064	0	0

No. 2.—WASTE •WEIR.

MEASUREMENTS.

	No.	Length.	Breadth.	Depth.	Solid Feet.		
Total solid feet excavation for } waste weir	• •				8,12,325		
Total solid feet excavation in moorum for waste weir							

Apron to Waste Weir. Excavating Foundation.

	No.	Length.	Breadth.	Depth.	Solid Feet.				
The wall at two ends		108	4	6	5,184				
For the pavement		60	108	4	25,920				
Outside of ditto		16	108+135	2	3,888				
Total solid feet of excavating in moorum, for the foundation of the apron to the waste weir									
Uncoursed Rubble Masonry in Foundation of the Apron.									
	No.	Length.	Breadth.	Depth.	Solid Feet.				
End walls	$_2$	$\frac{108+103}{2}$	4	6	5,064				
Under the pavement	1	108+103	60	3	18,990				
Small retaining wall, both sides of the apron	2	68	1.5	3	612				
Total solid feet of uncoursed rubble masonry in the foundation of the apron									
Rough Stone Pavement in Waste Weir, set on Edge in Lime.									

	No.	Length.	Breadth.	Square Feet.		
Between small retaining walls Outside of ditto ditto	1	60 16	100	6,000 · 1,944		
Total square feet of rough stone pavement, set on edge in lime						

ABSTRACT.

Quantity.		Rs.	a.	p.
8,12,325	Solid feet of excavating in moorum for waste weir, at Rs. 0-10-6 per 100 solid feet	5,330	11	1
34,992	Solid feet of excavating in moorum for foundation of the apron, at Rs. 0-10-6	·		
24,666	per 100 solid feet	229	10	1
7,944	per 100 solid feet	1,973	4	5
	on edge in lime, at Rs. 0-5-0 per square foot	9 499	v	Λ
	Total R ₁ 1	,		
	Contingencies, at 5 per cent.	500		
	Total amount for waste weirRs1	0,517	0	0

No. 3.—ARTIFICIAL CUT TO CARRY OFF THE FIRST MONSOON FLOODS.

	No.	Length.	Breadth.	Depth:	Solid Feet.
Total solid feet of ex- cavation in moorum in cut			• •		30,97,396-87
Total solid feet of excavati	_			- 1	30,97,396.87

ABSTRACT.

Quantity.

Rs. a. p.

30,97,396 Solid feet excavating artificial cut in moorum, at Rs. 0-10-6 per 100 solid feet . . }

20,326 10 6

Total . . Rs. | 20,326 10 6

Contingencies, at 5 per cent. | 1,016 5 3

Total amount for excavating artificial cut Rs. . 21,342 0 0

CONSTRUCTING MASONRY DAM ACROSS THE AMBEYGAUM NULLAH.

Excavating Foundation in Earth.

		No.	Length.	Breadth	Depth.	Solid Feet.
Ist Po	rtion from the lef	t	10	8.70	7·82+4·59	539·8 3
2nd	do.		3	8.70	$\frac{1.59 + 2.31}{2}$	90.04
3rd	do.		4	8.70	$\frac{2 \cdot 31 + 2 \cdot 28}{2 \cdot 31 + 2 \cdot 28}$	19.86
4th	do.		6	12.30	7-17+1-24	310.32
5th	do.		11	12.30	1.24	83.88
6th	do.		15	12.30	0.95	8 7·63
7th	do.		4	12:30	7-80+0-95	215.25
8th	do.		7	12.30	7.80 + 9.58	748.20
9th	do.		20	8.86	5.72+5.00	949.79
Total	solid feet excavat	ing founda	tion in	earth.		3 104:80

10 P

Excavating Foundation in Soft Rock.

	No.	Length.	Breadth.	Depth.	Solid Feet.
For foundation	1	43	12:30	2	1,057.80
Total solid feet excavating four	1,057.80				

Building Foundation of Uncoursed Rubble Masonry.

	No.	Length.	Breadth.	Depth.	Solid Feet.
1st Portion 2nd ditto 3rd ditto	1	17	8.70	2	295.80
2nd ditto	1	43	12:30	2	1,057.80
3rd ditto	1	20	8.86	2	354.40
Total solid feet uncoursed rub ation					

Building Dam of Coursed Rubble Masonry.

	No.	Length.	Breadth	Depth.	Solid Feet.
1st Portion	1	17	6.70+5	5 ·41	538.02
2nd ditto	1	43	10.3+5	10.30	3,388.18
1st Portion	1	20	$\frac{6.86 + 5}{2}$	5.72	678.39
Total solid feet of coursed rubb					

ABSTRACT.

Quantity.		Rs.	a.	P
3,104	Solid feet excavating foundation in earth,			
	at Rs. 0-4-0 per 100 solid feet	7	12	1
1,057	Solid feet excavating foundation in soft			
	rock, at Rs. 2 per 100 solid feet	21	2	2
1,708	Solid feet uncoursed rubble masonry in			
	foundation, at Rs. 8 per 100 solid feet	136	10	2
4,604	Solid feet of coursed rubble masonry in the			
	superstructure, at Rs. 9-8-0 per 100 solid			
	feet	437	6	0
	TotalRs	602	14	5
ntingenc	ies, at 5 per cent	30	2	3
ital amou	nt for the Ambeygaum Nullah danıRs	633	0	0

CONSTRUCTING MASONRY DAM ACROSS THE LAINDEE NULLAH.

Excavating Foundation in Earth.

e va aga ayuumaanaa			No.	Length.	Breadth.	Depth.	Solid Feet.
1st Por	rtion fron	n the left	 1	23	11.83	10-17+0-22	2,637.91
2nd	do.	do.	 1	16	11.83	8-55+8-00	1,686.48
3rd	do.	do.	 1	11	11.83	8:00+8:71	1,126.27
4th	do.	do.	 1	10	11.83	8-71+9-59 2	1,023-29
5th	do.	do.	 1	14	11.83	8.59+0.00	1,456.62
6th .	do.	do.	 1	9	17.63	18·46 + 15·69 2	2,709-29
7th	do.	ďο.	 1	17	17.63	15·09+13·48 2	4,371.27

CONSTRUCTING MASONRY DAM—(continued).

				No.	Length.	Breadth.	Depth.	Solid Feet.			
8th	Portion from	the left		1	14	17.63	13.48+10.00	2,897.66			
9th	do.	do.		1	9	17:63	10.00 + 20.19	2,395·12			
10th	do.	do.		1	20	12.5	11-19+11-33	$2,758\cdot70$			
	Total solid feet excavating foundation in earth										

Building Foundations of Uncoursed Rubble Masonry.

										No.	Length.	Breadth.	Depth.	Solid Feet.
1st P	ortion									1	74	11.83	0.54	427.72
2nd	do.										49	17:63	2.5	2,159.67
3rd	do.									1	49	15.63	2.5	1,914·6 7
4th	do.										49	13.63	2.5	`1,669·6 7
5th	do.										49	11.63	2.5	1,424.67
6th	do.									1	20	12.5	0.95	151.90
Total s	solid fe	eet 	un	co	ur: 	se(1 :	ru 	ЬЬ • •	ole ma	asonry i	n the for	ında- /	7,748:30

Building Dam of Coursed Rubble Masonry.

	No.	Length.	Breadth.	Depth.	Solid Feet.
Building dam	1	143	9:03 + 1:73	9.63	9,901.27
Total solid feet coursed rubble	9,901.27				

ABSTRACT.

Quantity.		Rs.	a.	р.
23,062	Solid feet excavating foundation in earth, at Rs. 0-4-0 per 100 solid feet		10	5
7,748	Solid feet uncoursed rubble masonry in the foundations, at Rs. 8 per 100 solid feet		13	5
9,901	Solid feet coursed rubble masonry in the superstructure at Rs. 9-8-0 per 100 solid feet	940	9	6
•	TotalRs	1,618	1	4
Conting	gencies, at 5 per cent	80	14	5
Total a	mount for the Laindee Nullah damRs	1,698	0	0

MASONRY DAM ACROSS THE KONDWEH NULLAH.

Excavating Foundation in Moorum.

	No.	Length.	Breadth.	Depth.	Solid Feet.
1st Portion	1	8	15.56	17:48 + 14:85	2,012-21
2nd do		1	15.56	14.85+4.00	733.26
3rd do	1	14	15.56	4.00+5.75	1,061.97
4th do	1	14	15.56	75+11·70 2	1,900.65
5th do	1	29	9.30	$\frac{5.0+7.11}{2}$	1,633.03

Total solid feet excavating foundation in moorum 7,341·12

Building Foundation of Uncoursed Rubble Masonry.

	No.	Length.	Breadth	Depth.	Solid Feet.
1st Portion	1	41	15.56	2 -	1,275.92
2nd ditto	1	41	15.26	2	1,275.92
Total solid feet of uncoursed rudation					

Building Dam of Coursed Rubble Masonry.

•	No.	Length.	Breadth.	Depth.	Solid Feet.
1st Portion	1	41	11:56+5.75 2 7:70+5.75	11·56 7·11	4,102·12 1,386·62
Total solid feet of coursed rubbi					

ABSTRACT.

Quantity.		Rs.	. 8.	р.
7,431	Solid feet excavating foundation in moorum, at Rs. 0-10-6 per 100 solid feet	48	2	9
2,551	Solid feet uncoursed rubble masoury in foundations, at Rs. 8 per 100 solid feet	204	1	વ
5,488	Solid feet coursed rubble masonry in superstructure, at Rs. 9-8-0 per 100 solid	204		v
	feet	521	5	9
	TotalRs	773	9	9
Contin	gencies, at 5 per cent	38	10	10
Total amo	ount for the Kondweh Nullah DamRs	812	0	0

MASONRY DAM ACROSS THE NAHAVEE-DURRA NULLAII.

Excavating Foundation in Moorum.

	No.	Length.	Breadth.	Depth.	Solid Fcet.
1st Portion	1	8	8.97	8·97+5·19	508.06
2nd do	1	5	8.97	5:19+4:0 2	206.08
3rd do	1	7	8.97	4.0 + 6.08	316:46
4th do		4	8.97	6.08+6.83	231.60
5th do	1	10	8.97	6-43+7-48	641.80
6th do	1	10	6.97	4.0+5.49	330.72
Total solid feet exca	vating found	lation in	moorui	n	2,234.72

·Building Foundation of Uncoursed Rubble Masonry.

	No.	Length.	Breadth.	Depth.	Solid Feet.
1st Portion	1	34	8.97	2	609.96
2nd do	1	10	8.97	2	179.40
3rd do	I	10	6.97	0.52	36.24
Total solid feet uncoursed rubl	ole m	asonry i	n found	ation	825.60

Building Dam of Coursed Rubble Masonry.

	No.	Length.	Breadth.	Depth.	Solid Feet.
1st Portion	1	34	4.07 + 2.5	4.97	631·14
2nd ditto	1	10	4.97+2.5 • 2	4.97	185.62
Total solid feet coursed rubble	masc	nry in	superstr	ucture.	816.76

Total solid feet coursed rubble masonry in superstructure.

ABSTRACT.

Quantity.		Rs.	a.	p.
	Solid feet excavating foundation in moorum, at Rs. 0-10-6 per 100 solid feet	14	10	6
	Solid feet of uncoursed rubble masonry in foundation, at Rs. 8 per 100 solid feet	66	0	0
810	Solid feet of coursed rubble masonry in superstructure, at Rs. 9-8-0 per 100 solid feet	77	, 8	3
	TotalRs			
Contin	gencies, at 5 per cent	7	14	. 6
Total amoun	nt for the Nahavee-Durra Nullah Rs	166	1	3

RECAPITULATION.

		Rs.	a.	p.
				_
Amount f	or excavating the cut to carry off the first floods	21,342	0	0
do.	Ambeygaum Nullah dam	633	0	0
d g .	Laindee Nullah dam	1,698	0	0
do.	Kondweh Nullah dam	812	0	0
do.	Nahavee-Purra Nullah dam	166	0	0
	Grand total for the artificial cutRs	24,651	0	0

No. 4.—INLET TOWER.

Excavating the Foundation for the Tower.

		No.	Lengt	h. Breac	lth.	Depth.	Solid Feet.	
Excavating foundation	cower	1	33	3	3	13	14,157	
Total solid feet exca		unda	tion i	n sand	, g	ravel, }	14,157	
Coursed	l Ru	bble .	Maso	nry i	n Four	ıda	tion.	
	No.	Lengt	h. Bi	eadth	Depth.	So	lid Feet.	Solid Feet.
1st portion from the base of the tower 2nd do. do 3rd do. do 4th do. do 5th do. do 6th do. do 7th do. do 8th do. do 9th do. do	1 1 1 1 1 1	33 31° 30° 29° 28° 27° 26° 25° 24°	· · · · · · · · · · · · · · · · · · ·	3 854 854 854 854 854 854 854	2 1 1 1 1 1 1 1		178·00 754·76 706·86 660·52 615·75 572·55 530·93 490·87 452·39	
Total			. .					6,962.63
Semisphere inverted. Portion of iron tube	1	18³		7 2530		1,8	526·81	
occupying the space	8:5+4:5	-	32	·7854		45.95		
Total deductions				• • •		•		1,572.76
Total solid feet coursed rubble masonry in the foundation of the tower								

Circular Invert on which the Tower rests.

	No.	Diameter	Breadth.	Depth.	Solid Fect.	Solid Feet.	
Taking the outer surface of the circular invert	1	18³	·5236 2		1526.81		
Total deduct.		••••				1526-81	
Semispherical portion		13 ^s	<u>·5236</u> 2		575.17		
tube	1	2.5+4	3²	.7854	22.97	5 98·1 4	
Total solid feet in semispherical invert of roughly-dress-							

Domed Floor on which the Steps rest, over the Filter.

	No.	Length.	Breadth.	Depth.	Solid Feet.	Solid Feet.
Taken as a solid mass.	,1	13²	·7854	2.75	356.01	
Total	• •					356.01
🗪 руст.						
Segmental portion .	1	13×3 -	— 1·25×	√2×1·25	2×5236=	29.86
Total solid feet in domed stone floor of cut-stone masonry						

Walls of Tower.

		•				
	No.		Length.	Breadth.	Height.	Solid Feet.
Walls of tower	1	1 56.5488+50		5+3	60	12,816.52
Total solid feet cut	-sto	ne :	masoury		• • • • • •	12,816.52
Steps in	the	Ins	ide of	Tower.		
	ı	So.	Length.	Breadth.	Depth.	Solid Feet.
Steps		14	2	1	1	44
No. of cut-stone steps in the	inte	rio	r of the	tower		44
Filling in with Dry Stone t	he I	Bas	c of the	Tower,	, used as	s a Filter.
	N	о.	Length.	Breadth	Depth.	Solid Feet.
1st portion		1	$\frac{13^{3}}{13^{2}}$	-5286 2 ·7854	1.25	575·17 165·91
Total solid feet filling in with	ı dr	y st	tone (an	ıygdaloi	d)	741.08
Filling in Fine o	and	Co	arse Sa	ind to I	Filter.	
	N	0.	Length.	Breadth.	Depth.	Solid Feet.
Base of tower (coarse sand).		1	13²	·7854	5.75	763-21
Upper part (fine sand)	•	1	13^{2}	· 7 854	5.25	* 696·8 4
Do. (do.)	\cdot	1	13 ×3-1	25×2 × 1·25	×·5236	29.86

Total solid feet filling in sand to the filter 1,489.91

Cornice to Tower at Top.

			No.	Diamet	er.	Running Feet.
Cornice of inlet tow	er			19	3.1416	59.69
Total running feet		59 ·69				
Pc	rtion	of To	ver abou	ve Corn	ice.	
	No.	Length.	Breadth.	Depth.	Solid Feet.	Solid Feet.
Wall	1	17:5	×3·1416×1·5>	(10-42	859:30	859:30
Doors, rectangular parts ,, circular parts. Large door, rectangular part. ,, circular part Total deductions.	3 3 1 1	6·2 8 A 25·1	1·5 rea 832 1·5 rea 328	6 1·5 4 1·5	108·0 28·27 48·0 37·69	221.96
Total solid feet cours tion of tower above		cornice	•••••			637:34
		Roof of	f Tower	•		
			NO 1	Slant leight.	Perimeter.	Square Feet.
Roof	••••	{	1 ∴	12·69	\$2 × 3·1416	} 438·53
Total square feet of d	ouble	tiled to	ak-wood	l roof		438.53

Iron Work.

No. Length. Diameter.

Tube taken ten feet outside the base of the tower	1	86.5	3′
Pipes by which the water from the reservoir enters the bottom of the tower	8 {	each 20	} 1'
Tubes in the walls of the tower, eight in number, in the circumference at each length	72 {	average 5	}
Gratings, eight in number, seven of 1 foot in diameter, and one of 3 feet in diameter			

ABSTRACT.

Quantities.		Rs.	a.	р.
14,157	Solid feet excavating foundation in sand, gravel, and water, at Rs. 2-8-0 per 100 solid feet		14	9
5,389	Solid feet of coursed rubble masonry in foundation of tower, at Rs. 9-8-0 per 100	บบบ	14	3
928	solid feet	511	15	3
000	lime in semispherical invert at Rs. 30 per 100 solid feet	278	6	4
326	Solid feet of cut-stone masonry in domed stone floor, at Rs. 0-9-0 per 100 solid feet	183	6	0

ABSTRACT—(continued).

Quantities.		Rs.	н.	p.
12,816	Solid feet of cut-stone masonry in walls of tower, at Rs. 45 per 100 solid feet (coursed rubble masonry faced with cut-stone on both sides)	5,767	*	2
44	Number of cut-stone steps in the interior of the tower, at Rs. 2-4-0 per step	99		0
741	Solid feet filling in with dry stone, to the base of the tower, at Rs. 2-8-0 per 100			U
1,489	solid feet	•	8	
$59\frac{1}{2}$	Running feet of cornice round the top of		U	U
637	the tower, at Rs. 5-6-0 per foot Solid feet of coursed rubble masonry in wall of circular portion above the cornice,	319	13	0
43 8	at Rs. 9-8-0 per 100 solid feet Square feet of roofing of double tiles, teakwood cut battens, and teak-wood cut rafters, at Rs. 29-8-0 per 100 square	60	8	2
1	feet	129	3	4
8.	centre of the tower	2,448	13	10
	tower	712	11	0
72	Iron tubes in the walls of the tower	611	12	4

ABSTRACT—(continued).

Quantities.		Rs.	a.	p.
7	Iron gratings, 1 foot diameter, at Rs. 3 cach	21	0	
	each	10	0	0
	TotalRs	11,593	3	2
Contin	geneies, at 5 per cent	579	10	6
	Total amount for inlet towerRs	12,172	0	0

No. 5.—GANGWAY.

Excavating Foundation.

•	No.	Length.	Breadth.	Depth.	Solid Feet.
For pier		1	1		í
Total solid feet excavating fo	unda	tion in	sand, g	ravel,	3,520

Filling in Foundation of Uncoursed Rubble Masonry.

	No.	Length.	Breadth.	Depth.	Şolid Feet.
Abutment in embankment, off-					
set of	1	13	7.0	1	91
Do. do	1	11	5.0	1	55
Centre pier, 1st offset	1	22	16	2	704
Do. upper offset	1	$\begin{array}{c} 3 & 0+13 \\ \hline 2 & \end{array}$	14+7	8	173.25
Total solid feet filling in found	latior	is of ur	coursed	rub-}	1,023-25

Superstructure of Piers and Abutment.

	No.	Length.	Breadth.	Depth.	Solid Feet.		
Centre pier, portion over foot-							
$ings \dots \dots \dots$	1	12	6	19.5	1,423.5		
Do. middle do	1	11	5	20 0	1,100.0		
Do. upper do	1	10	4	17.0	680.0		
Abutment in embankment	1	10	4	12.0	480.0		
Total solid feet in superstructure in pier and abutment							
of coursed rubble masonry.		-		1	3,683.5		

WOODWORK.

(The whole Framing taken.)

TEAKWOOD.

	No.	Length.	Breadth	Thickness.	Solid Feet.
Lower chord pieces (taken					
as one beam)	2	180′	14"	10"	350.00
Upper do. (do)	2	180'	15''	$7_2^{1''}$	281.25
Main braces	96	93′	$5\frac{1}{2}''$	5″	178.75
Counter braces	48	93'	5″	5"	81.25
Lateral braces	46	9'	8"	3"	69.00
Caps on pier	2	83'	14"	10"	17.01
Ditto.	2	113'	14"	10"	22.84
Caps on tower and abut-					
ment in embankment	4	113'	14"	10"	45.69
Struts	4	11'	14"	6"	25.66
Planking	1	180'	8"	$3\frac{1}{2}''$	420.00
End panels, posts	16	74′	5"	6"	24.16
Do. counter braces	4	71/4	5″	5"	5.03

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woodwork—(continued).

ł	No.	Length.	Breadth.	Thickness.	Solid Feet.
End panels, main braces Do. pieces to receive	8	71/4	5½"	5"	11.07
the feet of ditto	4	$3\frac{\circ}{3}'$	14"	4"	5.70
Side planking	2	180'	9"	2"	45.00
Solid feet	*				1,582.41
Add th for wastage					316.48
Total teak-wood, solid feet.					1,898.89
BAROOL WOOD.					
Tongues between timbers of					
lower chord	72	6"	$2\frac{1}{2}''$	10"	6.25
Tongues between timbers of		1	į		
upper chord	104	6"	$2\frac{1}{2}''$	71"	6.77
Lower chord, pieces at joints	İ	}			
of timbers	68	4'	3"	10"	56.66
Solid feet	69.68				
$\operatorname{Add} rac{1}{5} ext{th for}$	13.93				
Total baboo	ol-woo	od solid	feet		83:61

IRON WORK. Castings.

	No.	Length.	Breadth.	Depth.	Pounds Avoirdupois.
Lower chord angle blocks, each 45 lbs	46	16"	717	$14\frac{1}{8}''$	2,070.00
Upper chord angle blocks, each 40 lbs	66	16"	$7\frac{1}{2}''$	115"	1,840.00
Total lbs. of iron	3,910.00				

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Wrought Iron (round).

	No.	Length.	Total Length in Feet.	Size.	Tabular Number.	Ounces Avoirdupois
				dıa.		
Lateral brace bolts	24	8¾'	204'	1"	41.740	8,514.96
Bolts for chord pieces (lower chord)	104	1:7"	164.66	dıa. 1"	10.435	1,703·40
Bolts for chord pieces (upper chord)	104	1′7″	164·66	dia. 1" 2	10.435	1,703·40
Suspension bolts be-	100	81'	850°	dia. 1"	41.740	35,479.00
At the intersection of main and counter braces	52	147"	82.33	dia. 1/2"	10.435	859-11
Flat.			1			
Nuts for lateral brace	48	2"	8'	2"×1½"	159•436	1,275·48
Nuts for bolts to upper and lower chords	208	l ½"	26'	1½"×1"	79.718	2,072.66
Nuts for suspension bolts.	200	2"	33433	2"×1½"	159.436	5,314.00
Nuts at the intersection of main and counter braces	52	11,"	645	1½"×1"	79·718	518-16
Iron plates at top and bottom of suspension bolts	100	16"	133433	3½"×½"	93.004	12,400·22
Washers	284	21/2"	59.16	2½"×}"	33.216	1,965.05
Total weight in or	unces		• • • • • •	• • • • • • •	• • • • • • •	71,805.44
Total lbs. of wrou	ght-i	ron wor	k	• • • • • •		4,487.00
		Spikes	and Na	ils.		· ————————————————————————————————————

·	No.	Length.	Pounds.	
5-inch spikes	900 500	5" Sorts.	60·00 40·00	
Total lbs. weight of spikes and nails				

Labour.

	Days.	Total Days.
Carpenters, 1st sort	450	450
Do. 2nd sort	850	850
Carpenters, 1st sort	1750	1750

Scaffolding.

	Number.
Scaffolding (360 running feet)	1
Total No. of scaffolding	1

ABSTRACT.

Quantities.		Rs.	a.	p.
3,520	Solid feet excavating foundation in sand, gravel, and water, at Rs. 2-8-0 per 100			
	solid feet	88	0	0
1,023	Solid feet filling in foundation of uncoursed rubble masonry, at Rs. 8 per 100 solid			
	feet	81	13	5
3,683	Solid feet in superstructure of pier and abutment of coursed rubble masonry,			
	at Rs. 15 per 100 solid feet	552	7	2
1,898	Solid feet of teak-wood in woodwork, at Rs. 2-15-2 per foot	5,595	2	4

ABSTRACT—(continued).

Quantities.		Rs.	8.	р.
83	Solid feet of babool-wood in woodwork, at Rs. 0-12-0 per foot	62	4	0
3,910		0.2	-1	V
	per cwt	459	13	5
4,487	Pounds of wrought-iron in bolts, nuts,			
	washers, &c., at Rs. 6 per 28 lbs	961	8	0
100	Pounds of spikes and nails of sorts, at			
	Rs. 1 per 7 lbs	14	4	6
450	Days carpenters' hire, at Rs. 0-8-0 per day.	225	0	0
850	Days do, at Rs. 0-6-0 per day.	318	12	O
1,750	Days labourers' hire at Rs. 0-2-0 per day.	218	12	0
1	Scaffolding	72	0	0
	Total Rs 8	3,649	12	io
	Add contingencies, at 5 per cent	432	7	10
	Total amount for gangwayRs9		0	0

No. 6.—MASONRY AQUEDUCT MEASUREMENTS.

Excavating Trench for the Aqueduct.

	No.	Length.	Breadth.	Depth.	Solid Feet.
For that portion of the aqueduct, from the inlet tower to the mouth of the tunnel, on the Ambeygaum side	\ 	.	••••	, . , .	8,05,510

MASONRY AQUEDUCT, &c.—(continued.)

			No.	Length.	Breadth.	Depth.	Solid Feet.
duct f shaft N	rom the	of the aque- e chambered the distribu- in the camp	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\			•••	7,57,231
		excavating tro					15,62,741
1-5th 4-5ths	do. do.	taken in so taken in m	oft roc	ek n			3,12,548 12,50,193
				Total se	olid fect	• • • • • •	15,62,741

Excavation for the Air-Shafts of the Aqueduct.

		No,	Length.	Breadth.	Depth.	Solid Feet.
Air-shaft	s	66	6.5	6.5		5,577
Total soli	d excava	ation for the air-sha	fts of t	he aque	luct	5,577
1-5th	do.	taken in soft rock				1,115.4
4-5ths	do.	taken in soft rock taken in moorum			• • • • • •	4,461.6
		Total solid feet	ι			5,577.0

Building Aqueduct, Interior Measurement 18×15 inches, slab-top.

	No.	Length.	Breadth.	Depth.	Solid Feet.
Total length Feet 34,367.5					
DEDUCT:—					
Portion under the embank- ment Feet 335:0					
Raised portion ,, 1,147.5					
Portions included in				'	
the air-shafts ,, 396.0					
1,878.5			İ		
		ļ	1		
32,489:0					-
Sill of aqueduct	1	32,489	4	1	1,29;956.0
Sides of do	2	32,489	1	1.25	81,222.5
Top of do	1	32,489	$3\frac{1}{2}$	1	1,13,711.5
Sills of air-shafts	66	6.5	6.5	1	2,788.5
Superstructure of air-shafts	66	Four sides.	$1\frac{1}{2}$	8	14,256.0
Total solid feet building aqu	oduo	t of go	ureod m	ıbble)	

Covering Mouths of Air-Shafts with Stone Slabs set in Lime.

	No.	Length.	Breadth.	Depth.	Square Feet.		
Mouths of air-shafts					1,056		
Total square feet of covering mouths of air-shafts with stone slabs set in lime							

Building Aqueduct, Interior Measurement 15" × 18". Portion passing under the Embanhment.

	No.	Length.	Breadth.	Depth.	Solid Feet.	Total Solid Feet.
Filling in foundation Superstructure, tak-	1	335	4	1	1,340.0	
en as a whole	1	335	$3\frac{1}{2}$	3	3,517.5	4,857.5
Open portion of						
aqueduct	1	335	1.5	1.25	628.125	
• Do. do:	1	335	1.2 ×	· · *7854	295.997	·
			,			924.12
Total solid feet of c that portion of the bankment, with a	he ac	Ineduct	passing	under	the em-	3,933·38

Raising a Portion of the Aqueduct near the Distribution Reservoir.

•	No.	Length.	Breadth.	Depth.	Solid Feet.
Excavating foundation	1	1,147.5	4.5	1:5	7,745.62
Total solid feet excavating	7,745.62				

Building Raised Portion of Coursed Rubble Masonry.

	No.	Length.	Breadth	Depth.	Solid Feet.
Foundation, same as ex-					7 745.60
cavation above		4.0		4.40	7,745·62 404·80
1st portion		46	4	2 4·40+8·86	
2nd do		50	4		1,326.00
3rd do		50	4	8.72+8.86	1,758.00
4th do		100	4	$\frac{11\cdot 18 + 8\cdot 72}{2}$	3,980.00
5th do		130	4	15:95+11:18	7,053.80
6th do		53	4	$\frac{13.78+15.95}{2}$	3,151.38
7th do		95	4	$\frac{7.50+13.78}{2}$	4,043:20
8th do		$52\frac{1}{2}$	4	2 25+7·50 2	1,023.75
9th do		9 7	4	$\frac{9.42 + 9.25}{2}$	905.98
10th do		93	4	$\frac{6.46 + 2.42}{2}$	1,651.68
11th do		32	4	6.40+6.46	823.04
12th do		50	4	$\frac{5.34 + 0.40}{2}$	1,174.00
13th do		50	4	6.72+5.91	1,206.00
14th do		46	4	$\frac{11\cdot13+6\cdot72}{2}$	1,642.20
15th do		54	4	$\frac{11\cdot13+13\cdot57}{2}$	2,667.60
16th do		44	4	$\frac{9.19 + 13.57}{2}$	2,002.88
17th do		56	4	0.14+0.19	1,716.96
18th do		49	4	$\frac{2\cdot 25 + 6\cdot 11}{2}$	$822 \cdot 22$
Total solid feet					45,099:11
Deduct,—opening of		1,147.5	1.5	1.25	2,151:56
Total solid feet coursed rubble masonry in raising aqueduct					

Filling in Trench over Aqueduct.

	Arca.	Solid Feet.
Solid feet of excavation in trench as above Deduct,—solid feet of masonry work of	1	15,62,741
the aqueduct, taken as area		4,22,357
Total solid t	11,40,384	
Half do		5,70,192
Solid feet of half filling into trench		5,70,192

ABSTRACT.

, Quantities.		Rs.	a.	p.
3,12,548	Solid feet of excavating trench for masonry aqueduct in soft rock, at Rs. 2 per 100			
12,50,193	solid feet	6,250	15	4
	100 solid feet	8,204	6	3
1,115	Solid feet of excavating bottoms of air- shafts in soft rock, at Rs. 2 per 100 solid feet	22	1	a
4,461	Solid feet of excavating bottoms of airshafts in moorum, at Rs. 0-10-6 per 100	22	7	J
	solid feet	29	4	4
3,41,934	Solid feet of building aqueduct of coursed rubble masonry (18 x 15 inches interior measurement), at Rs. 10-8-0 per			
13 г	100 solid feet	5,903	1	1

ABSTRACT—(continued).

Quantities.	Rs.	a.	p.				
1,056 Square feet covering mouths of air-shafts with slab stones set in lime, at Rs. 15-10-0 per 100 square feet	165	0	0				
3,933 Solid feet of building aqueduct of coursed rubble masonry (18" × 15" inches interior measurement, arched head), being the portion passing under the embank-	•						
ment, at Rs. 10-8-0 per 100 solid feet. 7,745 Solid feet excavating foundation in moo-	412	15	5				
rum for the raised portion of the aqueduct, at Rs. 0-10-6 per 100 solid feet 42,947 Solid feet building raised portion of the	50	13	2				
aqueduct of coursed rubble masonry, at Rs. 10-8-0 per 100 solid feet	4,509	6	1 1				
5,70,192 Solid feet filling in trench over aqueduct at Rs. 0-1-6 per 100 solid feet	534	8	10				
Total	56,082 2,804		1 2				
Total amount for portion of masonry aqueduct 15" × 18" inches interior measurementRs 58,886 0 0							
TUNNEL.							
$oldsymbol{Excavation}.$							
No. Length. Breadth. De	epth. Soli	d Fe	et.				
Shaft at the mouth of the tun- nel on the Ambeygaum side, No. 8 of Plan No. 7 14 14	31 11 <u>,</u> 1	956·	00				

TUNNEL—(continued).

Excavation.

	No.	Length.	Breadth.	Depth.	Solid Feet.
Air-shaft of tunnel, No. 7 of		374			
Plan No, 7, 1st portion	1	dia. 82	· 7 854	20	1,005.31
Do. 2nd do	1	42	· 7 854	83	1,043.01
Air-shaft of tunnel, No. 6 of					
Plan No. 7, 1st portion	1	82	·7854	20	1,005.31
Do. 2nd do	1	4^2	.7854	74.5	936.19
Shaft at the mouth of the tun-					
. nel on the Duncowree side,			•		
No. 5 of Plan No. 7	1	14.5	14.5	65.75	13,823.93
Tunnel. Portion revetted for					
a length of 20 feet from the					
mouths at each end. Fig. 3			arca		
of Plan No. 7	2	20	39.38) . (1,575.20
Tunnel. Portion unrevetted.				77.	
Fig. 2 Plan No. 7 (2,78112-				>62	
Two shafts. Two revetted parts.	_		area	000	1
$(\overline{14+14\frac{1}{2}} -20-20 =2713]$	1	2713	21.89		59,387·57
Total solid feet excav	ation	for tun	nel		90,732.52
Total solid feet excavating tunn	el in	rock	••••		60 962:77
Total solid feet excavating shafe	ts to				ŕ
of the rest)			. , ,		9,923.25
Total solid feet do.			in hard		
rum (2-3rds of the rest)	•]	19,846.50
Total solid feet 90,732.52					

Coursed Rubble Masonry in Entrance and Air-Shafts and Revetment to Mouths of Tunnel for a Length of 20 Feet.

	No.	Lengt	Breadth. Depth.	Solid Feet.
Entrance to tunnel. Shaft on				
the Ambeygaum side. No. 8				
of Plan No. 7. 1st portion	1	36		900.00
2nd portion	1	35	4.75	831.25
3rd do	1	34	4 5	765.00
4th do	1	33	4.25	701.25
5th do	1	32	4.0	640.00
6th do	1	31	3.75	581.25
7th do	1	30	3.2	525.00
8th do]	29	3.25	471.25
9th do	1	28	3.0	420.00
10th do	1	27	2.75	371.25
11th do	1	26	2.5	325.00
12th do]	25	2.25 6	337:50
13th do	1	24	2.0 4	192.00
Entrance to tunnel. Shaft on				
the Duncowree side. No. 5				
of Plan No. 7. 1st portion	1	37		971.25
2nd portion	1	36	5.0	900.00
3rd do	1	35	4.75	831.25
4th do	1	34	4.5	765.00
5th do	1	33	4.25	701.25
6th do	1	32	4.0	640.00
7th do	1	31	3.75	581.25
8th do	1	30	3.5	525.00
• 9th do	1	29	3.25	471.25
10th do	1	28	3.0	420.00
11th do	1	27	2.75	371.25

Coursed Rubble Masonry in Entrance, &c.—(continued.)

		No.	Length.	Breadth.	Depth.	Solid Feet.
12th portion		ı	26	2.5	5	325.00
13th do.		1	25	2.25	4.75	267.18
14th do.	•••••	1	24	2.0	4.0	192:00
Masonry wo	ouths of tunnel rk to the mouths ts, Nos. 6 and 7 of		20	30· 21· 17·	88	699·60
Plan No. 7	· i	2	18.849	2	24	1,809.50
	to tunnel, and revole of 20 feet Door	• • • •	g tunne Shafts.	• • • • • •	distance	17,531·53
						No.
Doors of to	eak-wood plank ba	ıtten				4
Total N	o. of teak-wood pl	ank l	batten d	oors to s	shafts	4
	Λ	BSTR	ACT.			
Quantities.					1	Rs. a. p.
60,962	Solid feet excava Rs. 12 per 100	~		in rock,	· ·	315 7 0
9,923	Solid feet excava shafts to tunne	ting	entrance	e and ai	r-	

100 solid feet

396 14 8

ABSTRACT—(continued).

Quantities.		Rs.	a.	p.
19,846	Solid feet excavating entrance and airshafts to tunnel in hard moorum, at Rs. 0-14-0 per 100 solid feet		10	5
17,531	Solid feet of coursed rubble masonry in entrance and air-shafts to tunnel, and			
	the revetment to the tunnel for a length of 20 feet at each mouth, at Rs. 15 per 100 solid feet		10	4
4	No. of doors to mouths of shafts of teak-wood plank batten, at Rs. 16 each door	64	. 0	0
	TotalRs	10,579	10	<u> </u>
Conting	encies, at 5 per cent	528	15	8
Total an	nount for tunnel	11,108	0	0

Masonry Aqueduct $4\frac{1}{2}' \times 3'$ Interior Measurement, with Arched Head.

MEASUREMENTS.

Excavation of Trench for Aqueduct.

	No.	Length.	Breadth.	Depth.	Solid Feet.
For that portion of the aqueduct from the mouth of the tunnel on the Duncowree side to the first chambered shaft, that is, between Nos. 1 and 5 of Plan No. 8	>.	••••	••••	••••	21,56,874
Solid feet of excavati	ng tr	ench for	aquedu	ıct	21,56,874

Masonry Aqueduct $4\frac{1}{2}' \times 3'$ —(continued).

Solid Feet.

1-10th solid	feet o	of excavation	taken in	ı rock	 2,15,687
1-10th	(do.	taken ir	ı soft rock	 2,15,687
4-5ths	(do.	taken in	moorum	 17,25,500

Total solid feet.. 21,56,874

Excavating Bottom of Chambered Shafts, below Sill of Aqueduct.

No.	Length.	Breadth.	Depth.	Solid Feet.
1	16	10.5	2	336.00
1	15.5	10.5	2	310.00
1	14.5	9.5	2	275.50
1	13.5	8.5	2	229.50
4	6			
	1 1 1 1 n in	1 16 1 15·5 1 14·5 1 13·5 n in rock	1 16 10·5 1 15·5 10·5 1 14·5 9·5 1 13·5 8·5 n in rock for bott	1 16 10.5 2 1 15.5 10.5 2 1 14.5 9.5 2

Building Chambered Shafts Nos. 1, 2, 3, and 4, of Coursed Rubble Masonry faced with Cut-Stone.

	No.	Length.	Breadth.	Depth.	Solid Feet.
No. 4 of Plan No. 7. Foundation	1	16	10.5	1	168.00
foundation		37	3.5	5	647.50

100

Building Chambered Shafts, &c.—(continued.)

	,			No.	Length.	Breadth.	Depth.	Solid Feet.
2nd po	rtion of	side w	alls, &c.	1	36	3.25	5	585.00
3rd	do.	do.		1	35	3.0	5	525.00
4th	do.	do.		1	34	2.75	5	467.50
5th	do.	do.		1	33	2.5	5	412.50
6th	do.	do.		1	32	2.25	5	360.00
7th	do.	do.		1	31	2.0	3	186.00
Divisio	n wall			1	3	2.5	33	247.50
No. 3 c	of Plan	No. 7.	Founda-					
tion				1	15.5	10.0	1	155.00
1st por	tion of	side wa	lls above					
-				1	36	3.25	5	585.00
2nd	do.	do.		1	35	3.0	5	525.00
3rd	do.	do.		1	34	2.75	5	467.50
4th	do.	do.		1	33	2.5	5	412.50
5th	do.	do.		1	32	2.25	5	360.00
6th	do.	do.		1	31	2.0	5	310.00
Divisio	n wall.			1	3	2.5	30	225.00
No. 2	of Plan	No. 7.	Founda-	*				
tion				1	14.5	9.5	1	137.75
1st por	tion of	side wa	lls above					
foun	dation			1	34	3.0	5	510.00
2nd	do.	do.		1	33 .	2.75	5	453.75
3rd	do.	do.		1	32	2.5	5	400.00
4th	do.	do.		1	31	2.25	3.75	261.56
5th	do.	do.		1	30	2.0	3.0	180.00
Divisio	on wall			1	3	2.5	21.75	163.12
No. 1	of Plan	No. 7.	Founda-					
· tion				1	13.5	8.5	1	114.75
1st po	rtion of	side wa	lls above					
four	ndation.			1	32	2.5	5	400.00

Building Chambered Shafts, &c.—(continued.)

	No.	Length.	Breadth	Depth.	Solid Feet.
2nd portion of side walls, &c. 3rd do. do	1	31 30 2	2.25 2.0 3	5 4 14	348·75 240·00 84·0
Total solid feet of coursed r cut-stone to four chambere					

Building Aqueduct, Arched Head Interior Measurement $3' \times 4\frac{1}{2}'$.

No.	Length.	Breadth.	Depth.	Solid Feet.
1	3,190·5 3,910·5	1·5 length of arc 6·283 area	1 3 1	25,418·25 35,194·50 24,569·67 11,731·5
		_		
	2 1 2 ma	3,910·5 2 3,190·5 1 3,910·5 2 3,910·5 2 3,910·5 e masonry, in	3,910·5 6·5 2 3,190·5 1·5 1 3,910·5 6·283 2 3,910·5 1·5 2 masonry, including	3,910·5 6·5 1 2 3,190·5 1·5 3 1 3,910·5 6·283 1 area 1

Filling in Trench over Aqueduct.

	No.	Length.	Breadth.	Depth.	Solid Feet.	Total Solid Feet.
Solid feet of excavation, as above	••	••••	••••		21,56,874	
14 P	1		l			21,56,874.0

Filling in Trench over Aqueduct—(continued).

	No.	Length.	Breadth.	Depth.	Solid Feet.	Total Solid Feet.
Brought forward				••••		21,56,874.0
Deduct :						
Solid feet of masonry work of the aqueduct. (Taken as a rectangle). Do. do. No. 1 cham-	1	3,910.5		rca 9	1,52,509·5	
bered shaft	1	13	ŝ	9.	936.0	
Do. do. No. 2 do	ı	14	9	17.75	2,236.5	
Do. do. No. 3 do	1	15	9.5	19.	2,707.5	
Do. do. No. 4 do	1	15	10	29.	4,350.0	1,62,739.5
				Total se	olid feet	19,94,134.5
				Half	do	9,97,067.25
Total solid	feet o	f half-filli	ng in tren	ch over a	queduct	9,97,067.25

Doors.

	No.
	-
Doors of teak-wood plank batten $3' \times 3'$	8
Total No. teak-wood plank batten doors $3' \times 3' \dots$	8

ABSTRACT.

Quantities.	Rs.	a.	р.
2,15,687	Solid feet excavating trench for aqueduct		
, ,	in rock, at Rs. 4 per 100 solid feet 8,627	7	8
2,15,687	Solid feet excavating trench for aqueduct		
	in soft rock, at Rs. 2 per 100 solid feet. 4,313	11	10

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ABSTRACT—(continued).

Quantities.	Rs.	a.	р.			
17,25,500 Solid feet excavating trench for aqueduct in moorum, at Rs. 0-10-6 per 100 solid feet	11,323	9	6			
aqueduct in rock, at Rs. 4 per 100 solid feet	46	0	7			
faced with cut-stone in chambered shafts, at Rs. 30 per 100 solid feet Solid feet of building aqueduct $(3' \times 4\frac{1}{2})'$	2,979	9	7			
arched head) of coursed rubble masonry, at Rs. 15 per 100 solid feet 14,536 9,97,067 Solid feet filling in trench over aqueduct, at Rs. 0-1-6 per 100 solid feet 934						
TotalRs	42,834	2	4			
Contingencies, at 5 per cent	2,141		3			
Total amount for aqueduct with head of arched masonry	44,975	0	0			
RECAPITULATION.						
	Rs.	a.	p.			
Total amount for the portion of masonry aqueduct; rectangular section 18"×15" interior measurement, including that passing under the embank-						
ment, including that passing assets	58,886	0	0			

RECAPITULATION -- (continued).

	Rs.	a.	p.
Total amount of tunnel	1 08	0	0
Total amount for the portion of the masonry aqueduct, circular head $4\frac{1}{2} \times 3$ feet interior measure-			
ment	44,975	0	0
Total amount for the masonry aqueduct, including tunnel		0	Λ.
tunnel	1,14,969	0	()

IRON CONDUIT PIPE 13 INCHES DIAMETER.

MEASUREMENTS.

Excavating the Trench for the Iron Conduit Pipe.

			No.	Length.	Breadth.	Depth.	Solid Feet.
trench, tower reserve cepting	, from to the d ir in the g that p	tion of the the inlestification camp, exortion octunnel .	t				3 6,43,179
		excavatin					26,43,179
1-10th so 1-10th 4-5ths	do.	do.	ta	ken in s	oft rock		
				Total	l solid fe	et	26,43,179

Building Chambered Shaft No. 3 of Plans 7 and 8. Excavating Foundation.

•	No.	Length.	Area.	Solid Feet.
No. 3 chambered shaft	1	15:5	430.5	6,672.75
Deduct:—Portion included in the trench	1	15.5	262.5	4,068.75
Total solid feet excavating for the foundation of No. 3 chambered shaft				
1-10th solid feet excavation taken in	rock			260.4
1-10th do. do. taken in	soft r	ock		260.4
4-5ths do. do. taken in	moor	um		2,083.2
•	Tota	l solid f	eet	2,604:0

Coursed Rubble Masonry faced with Cut-Stone in No. 3, as above.

				No.	Length.	Breadth.	Depth.	Solid Feet.
					15.5	10	1	155.0
1st port	tion of	side wall	s above					
found	lation.			1	36	3.25	5	5 85·0
2nd	do.	do.		1	35	3.0	5	525.0
3rd	do.	do.		1	34	2.75	5	467.5
4 h	do.	do.		1	33	2.5	5	412.5
5th	do.	do.		1	32	$2 \cdot 25$	5	360.0
6th	do.	do.		1	31	2.0	5	310.0
Division	n wall			1	3	2.5	30	$225\cdot0$
Total solid feet coursed rubble masonry faced with cut-								
stone in chambered shaft No. 3								3,040.0

Iron Pipe 13 Inches Diameter.

*	Length.	Running Feet.
Iron conduit pipe 13 inches diameter	27,343	27,343
Total running feet iron conduit pipe 13 inches	diameter	27,343

Filling in Trench over Pipe.

	Solid Fcet.
Same as excavating	26,48,179
Total solid feet filling in trench over iron conduit pipe.	26,43,179

ABSTRACT.

Quantities.		Rs.	n.	p.
con	feet excavating trench for iron duit pipe in rock, at Rs. 4 per 100	10,572	10	10
con	feet excavating trench for iron duit pipe in soft rock, at Rs. 2 per	7.000		~
	solid feet	5,286	5	Э
con	feet excavating trench for iron duit pipe in moorum, at Rs. 0-10-6 100 solid feet	13,876	11	1
	feet excavating foundation of mbered shaft No. 3 in rock, at			
	4 per 100 solid feet	10	6	4

${\tt ABSTRACT--} (continued).$

Quantities.		Rs.	a.	p.
260 Solid feet excavating foundati	on of			
chambered shaft No. 3 in soft				
at Rs. 2 per 100 solid feet		5	3	2
2,083 Solid feet excavating foundation				
chambered shaft No. 3 in moort				
Rs. 0-10-6 per 100 solid feet .			10	8
3,040 Solid feet coursed rubble masonry				
with cut-stone, at Rs. 30 pe				
solid feet		912	0	0
27,343 Running feet of iron conduit pi	•			
inches in diameter, at Rs. 4-13-				
running foot; trenching, &c. n			1.4	9
cluded		1,52,809	14	3
duct, at Rs. 0-1-6 per 100	-			
feet		2,477	15	8
		•		
Total				5
Contingencies, at 5 per cent		8,501	ა	10
Total amount for iron conduit pipe]	Rs	1,74,326	0	0
RECAPITULATION.				
		Rs.	8.	<u>. </u>
	-			
Total amount for the iron conduit pipe	- 1	1,74,326		0
Total amount for the tunnel, as before	• • •	11,108	0	0
Total amount for the iron conduit pipe, includ	ing		·	-
tunnelRs	-1	1,85,434	0	0

No. 7.—DISTRIBUTION RESERVOIR. TWO DAYS' SUPPLY.

MEASUREMENTS. *

Excavation.

			22.000	00000			
Section	ı on D.	E. F. G. i	n the cer	itre.			Solid Feet.
					30	28.5	
2nd	do.		4.2+1.0	×	12	36.6	
3rd	do.		$\frac{7\cdot 2 + 1\cdot 2}{2}$	×	20	114.0	
4th	do.		$\frac{7\cdot4+7\cdot2}{2}$	×	13	94.9	
5 h	do.		$\frac{8 \cdot 0 + 7 \cdot 1}{2}$	×	27	207.9	
6th	do.		<u>0.8+8.0</u>		60	$528 \cdot 0$	
7th	do.		$\frac{10.5 + 9.0}{2}$	×	22.5	226.12	
8th	do.		$\frac{3.56+2.5}{2}$	×	12	36.36	
9th	do.		$\frac{0.75 + 3.56}{2}$	×	25.5	54.95	
10th	do.		$\frac{0.00 + 0.75}{2}$	×	20	7.5	
Section	n on th	e east side,					
1st	portion		$\frac{1.5+0.00}{2}$	×	23· 7	= 17.77	
2nd			3.8+1.5	×	12	= 31.80	
3rd	do.		$\frac{6.8 + 3.8}{2}$	×	20	= 106.00	
$4 \mathrm{th}$	do.		$\frac{7.00+6.8}{2}$	×	13	= 89.70	
5th	do.		$\frac{7.6+7.00}{2}$	×	27	= 197.10	
$6 \mathrm{th}$	do.	• • • • • •	$\frac{9.2 + 7.6}{2}$	×	60	= 504.00	
$7 ext{th}$	do.		$\frac{10.1 + 9.2}{2}$	×	22.5	= 217.12	
8th	do.	• • • • • • •	$\frac{3.16+5.1}{2}$	×	12	= 31.02	
9th	do.	• • • • • • •	0.35+3.16	×	25.5	= 44.75	
10th	do.		0·00+0·35	×	6.3	= 1.62	
Total area 2 2,575.71							
Mean area1,287.855							
		1.9					51,514 ·20
		-,-					01,011.00

Solid Feet.

Seci	ion on	the east sid	e, at 70 i	feet	from t	he o	entre.	
1st p	ortion		01+0.00	×	1.6	=	0.08	
2nd	do.		$\frac{2\cdot 4 + 0\cdot 1}{2}$	×	12		15.00	
3rd	do.		5.1+2.4		20	=	7 8·00	
4th	do.		5·6+5·4	×	13	===	71.50	
$5 ext{th}$	do.		$\frac{0.2+5.0}{2}$	×	27	===	159:30	
6th	do.		$\frac{7.8 + 6.2}{2}$		60	==	420.00	
7th	do.		H·7+7·9	×	22.5	==	185.62	
8th	do.		0.7+1.76	×	12	==	10.98	
9th	do.		0.00+1.76	×	15.9		13.99	
	•		or .				054.47	
Total area. 954·47								
Last area 1,240 88								
2,195·35								
Mean area. 1,097 665								
$\therefore 1,097.665 \times 30 =$								
		.:. 1	,097·665	×	30 =			$32,929 \cdot 95$
Section	takon					et f	rom the	32,929.95
		on the east	t side, at	136	3·56 fc			32,929.95
centr	e. Sa	on the east	t side, at	136 t, 9	3·56 fo 54·47	; .·.	954.47	ŕ
centr	e. Sa	on the east	t side, at	136 t, 9	3·56 fo 54·47	; .·.	954.47	32,929·95 63,529·52
centre × 66	e. Sa i·56 =	on the east	t side, at the las	136 t, 9	3·56 fc 54·47	; .·.	954:47	ŕ
centre × 66	e. Sa 5·56 = taker	on the east	t side, at the las	136 t, 9 	5.56 fo 54.47 	; . the	954:47	ŕ
centre × 66 Section 1st po	e. Sa 5·56 = taker ortion	on the east	t side, at the lass de, at 60	136 t, 9 fee	6.8	; the	954·47 e centre. 4·42	ŕ
centre × 66 Section 1st po	e. Sa 556 = taker ortion do.	on the east	t side, at the lass de, at 60 1.3 2 4.3+1.3 2 4.5+4.3	136 t, 9 fee × ×	3·56 fo 54·47 et from 6·8 20 13	;	954·47 e centre. 4·42 56·00 57·20	ŕ
centre × 66 Section 1st po 2nd 3rd	e. Sa 3.56 = a taker ortion do. do.	on the east	t side, at the las	136 t, 9 fee × ×	3·56 fo 54·47 et from 6·8 20 13	;	954·47 e centre. 4·42 56·00 57·20	ŕ
centre × 66 Section 1st po 2nd 3rd	e. Sa 5:56 = taker ortion do. do. do.	on the east	t side, at the lass de, at 60 1.3 2 4.3+1.3 2 4.5+4.3	136 t, 9 fee × × ×	3·56 fo 54·47 et from 6·8 20 13 27	; the = = = =	954·47 e centre. 4·42 56·00 57·20	ŕ

15 P

Solid Feet.

6th portion
$$\frac{7\cdot 6+6\cdot 7}{2}$$
 × $22\cdot 5$ = $160\cdot 87$
7th do. $\frac{0\cdot 00+0\cdot 00}{2}$ × 37 = $12\cdot 21$
Total area $774\cdot 30$
Central area ... $1,334\cdot 83$
 $2,109\cdot 13$
Mean area $1,054\cdot 56$
 $1054\cdot 565 \times 60$ = $63,273\cdot 90$

Section taken on the west side, at 110 feet from the centre

1st p	ortion	 $\frac{0.6}{2}$	×	3.1	0.93
2nd	do.	 3.0+0.6	×	20	42.00
3rd	do.	 3.8+3.6	×	13	48.10
4th	do.	 8.8+4.4	×	27	110.70
5th	do.	 $\frac{4\cdot 4 + 6\cdot 0}{2}$	×	60	312.00
6th	do.	 <u>6.9</u> +6.0	×	22.5	145.12

Total area . 658·85 Last area . 774·30

2 1,433.15

Mean area. 716.575

 $\therefore 716.575 \times 50 =$

35,828.75

Section taken on the west side, at 160 feet from the centre.

lst p	ortion	• • • • • • •	2.8	×	18.6	=	26.04
2nd	do.	• • • • • • •	$\frac{3.0+2.8}{2}$	X	13	=	37.70
3rd	do.	• • • • • • •	3.6+3.0	×	27	===	89.10

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Solid Feet.

2,63,727.84

4th portion
$$\frac{5\cdot 2+3\cdot 6}{2}$$
 × 60 = 264·00
5th do. $\frac{6\cdot 1+5\cdot 2}{2}$ × 22·5 = 127·12
Total area 543·96
Last area 656·15
1,200·11
Mean area 600·055
∴ 600·055 × 27·75 = 16,651·52

Total solid feet of excavation in bcd of reservoir 2,63,727.84

Portion between Retaining Wall of Reservoir and Parapet
Wall surrounding it.

No.	Length.	Breadth.	Depth.	Solid Feet.
Between retaining and surrounding wall $133\frac{1}{4} \times 2 +2 \cdot 5+ (2'4'' \times 2) + (10 \times 2) = \frac{294'8''}{169'8''} \dots$	464.33	8	average 7½	27,859·80
Foundation of wall surrounding reservoir	965·33	3	3	8,685·00
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	320.66	3	3	2,885:94
	Tota	l solid feet		39,430.74

Do. . as above.

Solid Feet.

1-3rd	excavating	reservoir, t	aken i	in h	ard	moorum,	
\mathbf{solid}	feet						1,01,052·86 2,02,105·72
2-3rds	do.	taken in roc	ek,	do.			2,02,105.72
							3,03,158.58

Filling in Foundation of Uncoursed Rubble Masonry.

	No.	Length.	Breadth.	Depth.	Solid Feet.
For retaining wall of reservoir. For wall surrounding reservoir.		320·66 965·00	1	3 3	2,885·94 8,685·00
Total solid feet filling in found masonry	11,570.94				

Building Retaining Wall of Reservoir.

			,		
	No.	Length.	Breadth.	Depth.	Solid Feet.
Retaining wall $(133^{\frac{3}{4}} \times 2)$ +		-			
$2.5 + (1.10 \times 2) \times 2 = 547' 4''$					
$165 \times 2 = \dots 330' 0''$					
$\overline{877' \ 4''} = 52' \ 0''$	}*1	$825rac{1}{3}*$	21/+13 2	7	10,591.77
Division wall $165-26=139$	1	139	2.5	7	2,432.50
Cistern walls 26+26+10+10					7
=72	2	72	3	7	3,024.00
Do. large portions	4	5	2	7	280.00
Do. small do	4	2	2	7	112.00
Steps	20	6	7 2	2	840.00
m . 1 11 0		'	<u>'</u>		17 000.07

Total solid feet superstructure of cut-stone masonry 17,280-27

Stone Pavement.

	No.	Length.	Breadth.	Square Feet.
Bottom of reservoir $\overline{133' 9'' + 2' 10''}$ + 1' 3" = 137' 10" the length, and 165' + 2' 10" = 167' 10" breadth.	$\bigg\}_2$	137′ 10″	167′ 10″	46,266·05
Total square feet of cut-	stone	pavemo	ent	46,266.05

Filling in Earth between Retaining Wall of Reservoir and Wall surrounding it.

•	No.	Length.	Breadth.	Depth.	Solid Feet.
North side between the two walls Ditto ditto cast and west.	1	300 185	8′ 3″ 8′ 3″	15	37,125·00 19,078·12
Total solid feet filling in earth of the reservoir and the wall					

Wall surrounding Reservoir, of Coursed Rubble Masonry.

	No.	Length.	Breadth.	Depth.	Solid Feet.
North end, lower portion: $133' \ 9'' \times 2 + 2' \ 6'' + 2' \ 4'' \times 2 + 1' \ 8'' \times 2 + 4'' = 304' \ 4'' \dots$	\			2′	1,420.22
Do. middle do	1	304′	2'	2'	1,216.00
Do. upper do		304'	1' 8"	12.07	6,115.46
East side, 1st portion		26′ 25	2' 4"	2/2	61.25
Do. 2nd do	1	31′31		2'	125.24

Wall surrounding Reservoir—(continued).

No. Length. Breadth. Depth. Solid Feet	No.	Length.	Breadth.	Depth.	Solid Feet.
--	-----	---------	----------	--------	-------------

East side	e, 3rd p	ortion		1	51'93	1' 8"	2.6	112.51
Do.	4th	do.		1	128′58	1′ 8″	1	214.30
Do.	5th	do.		1	189'66	1′ 8″	7	2,212.70
South sid	le wall		• • • • • • •	1	304'	1'8"	7.12	3,607.46
West sid	e, 1st p	ortion		1	27'	2' 4"	$\frac{2}{2}$	63.00
Do.	2nd	do.		1	27+38-25	2	2	130.50
Do.	3rd	do.		1	38-25+67-5	1' 8"	3	264.37
Do.	4th	do.		1	67.5+189.06	1′ 8″	3.52	754.33
Do.	5th	do.		1	189.66	1′ 8″	7 ·87	2,487.70
	To	tal soli	id feet				• • • •	18,785.04
Deduct—Doorways 2 4' 1' 8" 7'								93.33
Total sol	lid feet	of c	oursed r	ubble	e mason	ry in t	he wall	
Total solid feet of coursed rubble masonry in the wall surrounding the reservoir								

Filling in Earth to Slope.

	No.	Length.	Breadth.	Depth.	Solid Feet.					
North side	1	304	80	7 2	85,120.00					
East side	1	158.5	80	8.8	27,896.00					
West side	1	193	85+8	8-53+0-67	42,180·15					
Total solid feet filling in earth to slopes, east, west, and										
north sides					1,55,196·15					

Coping to the Wall surrounding Reservoir.

	No.	Length.	Running Feet.
Coping	1	975′4″	975′ 4″
Total running feet cut-stone coping to top ing wall of reservoir			

Doors.

	Number.
Doors teak-wood plank batten 7'×4'	2
Total number strong teak-wood plank batten doors to reservoir	2

ABSTRACT.

Quantity.		Rs	. а.	 p.
1,01,052	Solid feet excavating reservoir in hard moorum, at Rs. 0-14-0 per 100			
	solid feet	884	3	3
2,02,105	Solid feet excavating reservoir in rock, at Rs. 4 per 100 solid feet	8,084	3	2
11,571	Solid feet filling in foundation of un- coursed rubble masonry, at Rs. 8			
	per 100 solid feet	925	10	10
17,280	Solid feet of superstructure of cut-			
	stone masonry (coursed rubble masonry faced with cut-stone on the			
	inside), at Rs. 30 per 100 solid feet.	5,184	0	0

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ABSTRACT—(continued).

Quantity.		Rs.		ъ р.
46,266	Square feet of cut-stone pavement, at			
56,203	Rs. 31-8-0 per 100 square feet Solid feet filling in earth between the	14,573	12	7
	retaining wall of the reservoir, and the wall surrounding it, at Rs. 1-2-0			
18,691	per 100 solid feet	632	4	6
	in the wall surrounding the reservoir, at Rs. 9-8-0 per 100 solid feet.	1,775	10	3
1,55,196	Solid feet filling in earth to slopes, at Rs. 1-2-0 per 100 solid feet	1,745	15	3
975	Running feet of cut-stone coping to top	1,1 13	.,,	J
	of the wall surrounding the reservoir, at Rs. 1-4-0 per running foot.	1,218	12	0
2	Number of doors of teak-wood plank battened 7'×4', at Rs. 28 each	56	0	0
	Total Rs.	35,080	7	10
	Contingencies, at 5 per cent	1,754	0	4
	t for distribution reservoir to contain supply	36,834	0	0

DISTRIBUTION RESERVOIR.—ONE DAY'S SUPPLY.

MEASUREMENTS.

Excavation.

	Solid Feet.							
lst	portion	• • • • • • • •	$\frac{1.90+4.2}{2}$	×	12	==	36.60	
2nd	do.	• • • • • • • • •	$\frac{4\cdot 2 + 7\cdot 2}{2}$	×	20	==	114.00	

Solid Feet.

3rd	portion		$\frac{7 \cdot 2 + 7 \cdot 4}{2}$	×	13	_	94.90	
4th	do.		7-4+8-0	×	27	=	207.90	
5th	do.		8·0+9·0 2	×	60		528.00	
6th	do.		9-6+9-8	×	4	=	38.80	
7th	do.		5·50		3.75	_		
8th	do.		20·5 × 2			===	20.50	
(3)				O foot	Cuara	. 41.0	aontro	
		n on east si						
Ist	portion	• • • • • • •	$\frac{1.2 + 3.8}{5}$	×	12	=	31.80	į
2nd	, do.		$\frac{3.8+6.8}{2}$	×	20	==	106.00	
3rd	do.		$\frac{6.8 + 7.0}{2}$	×	13	=	89.70	
4th	do.		$\frac{7.0 + 7.6}{2}$. ×	27	==	197.10	
5th	do.		7:0+9:2	×	60	_	504.00	
6th	do.		$\frac{9 \cdot 2 + 9 \cdot 4}{2}$	×	4	==	37.20	
7th	do.		5·1	×	3:35	===	17.08	
$8 \mathrm{th}$	do.		$\frac{1.0\times20.5}{2}$			==	20.50	
			Т	otal a	rea	1	,003:38	
			•	JUL 1				ı
							,064.70	
					- 4		,032·35	
	Mean arc	ea 1,032·35	× 40 =	= soli	d feet		• • • • • • • •	41,294.00
Sectio	n taken	on the east	side, at	70 fee	et fro	m th	e centre.	
			0.1+2.4	×		=		
2nd	do.		2.1+5.4	×	20	==	78.00	
3rd	do.		5·4+5·6	×	13	==	71.50	
4th	do.		5.6+6.2	×	27	_	159.30	
5th	do.		6.2+7.8	×	60	==	420.00	
6th	do.		7.8+8.0	×	4	-	63.20	
	16 р		-				ŧ	

7th po 8th	rtion do.		1·95 0·2×20·5		5·5		10·72 2·05 819·77	Solid Feet.
			Li	ast a	rea		,003.38	
						1	,823·15 911·5 7 5	
		Mean area	. 011.578))		911.9/9	27,347.25
		mean area	1 911 976	, ^ (5 0		•	21,341 23
Section	taken	on east side		5 fee	t fror	n th	e centre.	
1st po	ortion		$\frac{2}{2}$	×	10.4		10.40	
2nd	do.		5+2		20			
	do.	• • • • • • •	$\frac{5+5\cdot 2}{2}$		13		66.30	
	do.	• • • • • •	2 2 5.2 + 5.8				148.50	
	do.		5.8+7.4				396.00	
	do.	• • • • • • • •	7·4+7·6 2	×			30.00	
	do.	• • • • • • • •	1.5	×	5.2	=	8.25	
8th	do.		$\frac{1.5 \times 20.5}{2}$			===	15.37	
			La	st a	rea		744·82 819·77	
							,564.59	
						~[1	$\frac{\cancel{7}82 \cdot \cancel{2}95}{\cancel{7}82 \cdot \cancel{2}95}$	
		Mean area	782-295	×41	.5 =			32,465.24
		on the west	side, at 6	80 fe	et fro	m th	ie centre. 4·42	
2nd	do.		2 1·3+4·3	×	20		56·00	
	do.	,,,,,,,	4.3+4.5	×				

Solid Feet.

4th portion
$$\frac{5\cdot 1+4\cdot 5}{2}$$
 × 27 = 129·60
*5th do. $\frac{6\cdot 7+5\cdot 1}{2}$ × 60 = 354·00
6th do. $\frac{6\cdot 9+6\cdot 7}{2}$ × 4 = 27·20
7th do. $\frac{6\cdot 9+6\cdot 7}{2}$ × 8·9 = 3·78
*Total area $\frac{636\cdot 87}{2}$ Last area $\frac{1,061\cdot 32}{2}$ $\frac{2[1,698\cdot 19]}{2}$ Mean area $\frac{849\cdot 09}{2}$... $\frac{849\cdot 09}{2}$ mean area × 60 = solid feet 50,945·40

Section taken on the west side, at 112.62 feet from the centre.

1st pe	ortion	• • • • • • •	<u>0.8</u>	×	3	=	0.90
2nd	do.		$\frac{2}{0.0+3.6}$	×	20	==	42.00
3rd	do.		3·0+3·H	×	13	=	48.10
4th	do.		3.8+4.1	×	27	==	110.70
5th	do.		$\frac{4\cdot 4 + 6\cdot 0}{2}$	×	60	-	312.00
6th	do.		60+62	×	4	=	24.40
$7 ext{th}$	do.	• • • • • • •	0-15	×		==	0.82

Total area.... 538·92 Last area 632·45 1,171·37

Mean area.... 585.685

Solid Feet.

Excavation	for	foundation	of	wall	${\bf surrounding}$	the
		reserv	oir	•		

Reservoir 201 +
$$(2' \ 4'' \times 2) + (10 \times 2) +$$

 $(1' \ 8'' \times 2) = 292 \times 2 = \dots 458$
Reservoir 110 + $(2' \ 4'' \times 2) + (10 \times 2) +$
 $134' \ 8'' \times 2 \dots 269'4$

727' 4"

$$727' \ 4'' \times 3 \times 3 = \dots$$
 6,546.00

Solid feet as above	 41,294.00
Do.	 27,347.25
Do.	 32,465.24
, Do.	 50,945.40
Do.	 30,818.74
Do.	 6,546.00

Total solid feet excavation 1,89,416:63

Total solid feet excavation....1,89,416.63

1-3rd excavating reservoir taken in hard moorum, solid feet 63,138·87 2-3rds do. do. taken in rock, solid feet 1,26,277·76

Total solid feet.... 1,89,416.63

Filling in Foundation of Uncoursed Rubble Masonry.

	No.	Length.	Breadth.	Depth.	Solid Feet.
Wall surrounding the reservoir, 1st offset	1	727′ 4″	3	3	6,546.00

Filling in Foundation—(continued).

	No.	Length.	Breadth.	Depth.	Solid Feet.		
Wall surrounding the reser-							
· . voir, 2nd offset	1	229′ 8″	2' 4"	7.77	4,163.85		
Do. do. east side	1	12	2' 4"	$\frac{2}{2}$	28.00		
Do. do. do	1	32	2' 4"	2.8	104.53		
Do. do. do	1	108′ 8″	. 2′ 4″	2 2	253.55		
Do. do. west side	1	15	2' 4"	3 2	52.50		
Do. do. do	1	45	2' 4"	3	157.50		
Do. do. do	1	229′ 8″	2' 4"	3.52+.87	1,178.95		
Total solid feet filling in foundation with uncoursed rubble							
masonry	<i>.</i>		• • • • • •	• • • • • •	12,484.88		

Building Retaining Wall of Reservoir.

No. Length. Breadth Depth. Solid Feet.

Retaining wall 201 + $(2' 4'' \times 2) \times 2 =411' 4''$					
Retaining wall 110 +					
$(2'4'' \times 2) \times 2 =229'4''$					
640.8	<u>}</u> 1	588′ 8″	2' 4"	1.75	2,403.72
Deduct—Cistern wall 52.0	İ				
588.8					
	j				
Retaining wall, 2nd offset	1	588′ 8″	2' 0"	1.75	$2,060\cdot33$
Do. 3rd do	1	588′ 8″	1′ 8″	1.75	1,716.94
Do. 4th do	1	588′ 8″	1' 4"	1.75	1,373.55
Cistern walls, 26 + 26 + 10 +					
10 = 72 in length	2	72	3	7	3,024.00

Building Retaining Wall—(continued).

	No.	Length.	Breadth.	Depth.	Solid Feet.
Cistern wall pieces at the ends					•
of steps	4	3	2	7	168.00
Steps	12	6	$\frac{7}{2}$	2	504.00
Total solid feet of superstruc	11,250.54				

Stone Pavement.

	No.	Length.	Breadth.	рерth.	Solid Feet.
Bottom of reservoir	1	206'8"	115′8″	1	23,904.44
Total square fee	23,904.44				

Filling in Earth between Retaining Wall of Reservoir and Wall surrounding it.

	No.	Length.	Breadth.	Depth.	Solid Feet.		
North side between the two							
walls		225'8''	8'3"	10'	18,617.50		
Do. east and west sides		134'7"	8'3"	7′ 0	15,544.37		
Do. south side		225'8''	6'3"	8'0	11,283.33		
Total solid feet filling in earth between the retaining wall							

Wall surrounding the Reservoir, of Coursed Rubble Masonry.

	No.	Length.	Breadth.	Depth.	Solid Feet.	
Wall surrounding the reservoir Deduct—Door-ways	1 2	727'4" 4	1'8" 1'8"	7 7	8,485·55 93·33	
Total solid feet of coursed rubble masonry in the wall surrounding the reservoir						

Filling in Earth to Slopes.

•	No.	Length.	Breadth.	Depth.	Solid Feet.
North side		229 138	122+00+180 3 G1	7·0 2 7·5	91,371·00 31,56 7 ·50
West side		138	58	8 2	32,016.00
Total solid feet filling in earth W. sides					

Coping to Wall surrounding the Reservoir.

	No.	Length.	Running Feet.
Coping	1	727'4"	727'4"
Total running feet of cut-stone coping to rounding wall of reservoir	4		

Doors.

	No.
Doors of teak-wood plank batten 7' × 4' to reservoir	2
Total No. of doors of teak-wood plank battened	2

ABSTRACT.

Quantities.		Rs.	a.	р.
63,138	Solid feet excavating reservoir in hard moorum, at Rs. 0-14-0 per 100 solid			
1 06 077	feet	552	7	3
1,20,277	Solid feet excavating reservoir in rock, at Rs. 4 per 100 solid feet	5,051	1	3
12,484	Solid feet filling in foundation with uncoursed rubble masonry, at Rs. 8 per	200		۸
11,250	Solid feet of superstructure of cut-stone masonry, at Rs. 30 per 100 solid feet	998	11	6
	(coursed rubble masonry faced with cut-stone on the inside)	3,375	0	0
23,904	Square feet of cut-stone pavement, at Rs. 31-8-0 per 100 square feet	7,529	12	1
45,445	Solid feet of filling in earth between the retaining wall of the reservoir, and the wall surrounding it, at Rs. 1-2-0	ŕ		
8,392	per 100 solid feet	511	4	1
,,,,,	the wall surrounding the reservoir, at Rs. 9-8-0 per 100 solid feet	797	3	10

ABSTRACT—(continued).

Quantities.		Rs.	8.	p.
1,54,954	Solid feet of filling in earth to slopes on the N. E. and west sides, at			
727	Rs. 1-2-0 per 100 solid feet Running feet of cut-stone coping to top of surrounding wall of reservoir, at		3	0
2	Rs. 1-4-0 per running foot No. of doors of teak-wood plank batten-		12	0
	ed 7' × 4', at Rs. 28 each	56	0	0
	TotalRs	21,523	7	8
•	Contingencies, at 5 per cent	1,076	2	9
Fotal amoun	t for distribution reservoir to contain			
one day's s	upply	22,599	0	0

No. 8.—CAMP DISTRIBUTION.

MEASUREMENTS.

Excavation for Cisterns $25' \times 18' \times 8'$.

	No.	Length.	Breadth.	Depth.	Solid Feet.	
Cistern	1	31	24	9	6,696.00	
Paving	1	$123\frac{1}{3}$	5	1	616.66	
Total solid feet excavation for cistern $25' \times 18' \times 8' \dots$						
½ of excavation for cistern, to	aken	in moor	um		3,656.33	
½ ditto ta	3,656.33					
Total solid feet						

Retaining Walls.

	No.	Length.	Breadth.	Depth.	Solid Feet.
1st portion	1	98	3	1	294.00
2nd do	I	96	2.5	2	480.00
3rd do	1	94'8"	$2^1_{\vec{6}}$	2	473.33
4th do	1	93'4"	15	2	342.22
5th do	1	92'	1.5	2	276.00
Total solid feet of retaining walls of coursed rubble mason- ry, faced with cut-stone on the inside					

Cut-Stone Masoury.

	No.	Length.	Breadth.	Depth.	Solid Feet.			
Parapet wall]	91′ 4″	1' 4"	2' 9"	334.88			
Total solid feet of cut-stone on both sides, including coping. 334.88								

Paving.

	No.	Length.	Breadth.	Square Feet.
In the cistern		25	18	450.00
parapet		124	6.2	806:00
Total square feet c	1,256.00			

Excavation for Small Cistern $15' \times 10' \times 8'$.

	No.	Length.	Breadth	. Depth.	Solid Fcet.		
Cistern		21	16	9	3 024:00		
Paving		94	5	1	470.00		
Total solid feet excavating for	smal	ll cistern	15' × 1	0'×8'.	. 3,494.00		
½ excavation for small cistern,	takeı	n in moo	orum		1,747:00		
do. do.	taker	n in soft	rock		1,747.00		
		Tota	al solid f	eet	3,494.00		
Reto	vining	ı Walls.					
•	No.	Length.	Breadth.	Depth.	Solid Feet.		
1st portion	1	62	3	1	186.00		
2nd do	1	60	$2\frac{1}{2}$	2	300.00		
3rd do	1	58′ 8″	$2^{\scriptscriptstyle 1}_{\scriptscriptstyle ec{0}}$	2	254.22		
4th do	1	57′ 4″	15	2	210.22		
5th do	1	56′	$1\frac{1}{2}$	2	168.00		
Total solid feet retaining walls faced with cut-stone on the				•	1,118.44		
Cut-Stone Masonry.							
	No.	Length.	Breadth.	Depth.	Solid Feet.		
Parapet wall	1	55′ 4″	l' 4"	2' 9"	202.88		
Total solid feet of cut-stone	on	both si	des, inc	luding			

Paving.

	No.	Length.	Breadth	Square Feet.
In the cistern		15 88	10 6·5	150 572
Total square feet cut-stone	pavi	ing		722

Distribution Pipes.

	Length.	Running Feet.
Iron pipes 5-inch diameter		
Total running feet 5-inch iron pipes Do. 4-inch iron pipes		23,585 23,585

ABSTRACT.

Quantities.		Rs.	a.	р.
3,656	Cistern $25' \times 18' \times 8'$. Solid feet excavation for cistern in moo-			
	rum, at Rs. 0-10-6 per 100 solid feet.	23	15	10
3,656	Solid feet excavation for cistern in soft rock, at Rs. 2 per 100 solid feet	7 3	1	11
1,865	Solid feet retaining walls to sides of cisterns of coursed rubble masonry, faced with cut-stone on the inside, at			
335	Rs. 30 per 100 solid feet Solid feet of cut-stone masonry on both	559	8	0
	sides, including coping for parapet, at Rs. 35 per 100 solid feet	117	4	0

${\tt ABSTRACT--} (continued).$

p.	a.	Rs.	Quantities.	Quantiti
			.1,256 Square feet of cut-stone paving to cistern, at Rs. 31-8-0 per 100 square	.1,25
2	10	395	•feet	
11	7	,169	TotalRs	
7	7	5 8	Contingencies, at 5 per cent	
		205	Total amount for the cistern 25' × 18'	
_0 	0	,227	× 8′	
			Cistern $15' \times 10' \times 8'$	•
			1,747 Solid feet of excavation for cistern in moorum, at Rs. 0-10-6 per 100 solid	1,74
5	7	11	feet	
			1,747 Solid feet excavation for cistern in soft	1,74
0	15	34	rock, at Rs. 2 per 100 solid feet Solid feet of retaining walls to sides of cistern, of coursed rubble masonry, faced with cut-stone on the inside,	1,118
4	6	335	at Rs. 30 per 100 solid feet	
9	0	71	Solid feet of cut-stone masonry on both sides, including coping for parapets, at Rs. 35 per 100 solid feet	20:
			722 Square feet of cut-stone paving to cisterns, at Rs. 31-8-0 per 100 square	722
0	6	227	feet	
4	4	680	$\operatorname{Total}\ldots\operatorname{Rs}.$	
3	0	34	Contingencies, at 5 per cent	
0	0	714	Total amount for the cistern $15' \times 10' \times 8'$	Total

ABSTRACT.

Quantities.		Rs.	а.	p.
23,585	Running feet of 5-inch iron pipes, including trenching, hydrants, &c.,		•	
	at Rs. 1-7-8 per running foot		2	4
23,585	Running feet of 4-inch iron pipes, in-			
	cluding trenching, hydrants, &c., at			
	Rs. 1-2-7 per running foot	27,392	15	11
	Total, Rs	62,279	2	3
	Contingencies, at 5 per cent	3,113	15	3
Total amou	ınt for camp distribution pipesRs	65,393	• ()	0

RECAPITULATION OF THE CAMP DISTRIBUTION.

	Rs.	a.	p.
Cisterns 24 in No., at Rs. 1,227 each	29,448	0	0
Cisterns 12 in No., at ,, 714 each		0	0
fron pipes for the camp distribution			
Total amount for the camp distribution	1,03,409	0	0

GENERAL RECAPITULATION OF THE ESTIMATE.

No.	•	•	Rs.	a.	р.
1	The embankment	1,52,	064	0	0
2	The waste weir	10,	51 7	0	0
3	The artificial cut to carry off the first floods.	24,	651	0	0
4	The inlet tower	12,	172	0	0

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GENERAL RECAPITULATION—(continued).

No.		Rs.	a.	р.
5	The gangway	9,082	0	0
6.	The masonry aqueduct and tunnel	1,14,969	0	0
,,	The iron conduit pipe and tunnel	1,85,434	0	0
	The distribution reservoir, to contain two days' supply	36,834	0	0
	day's supply	22,599	0	0.
8	The camp distribution	1,03,409	0	0

(Signed) Philip L. Hart, Captain, Engineers, on Special Duty.

Camp Poona, 23rd October, 1857.